

**U.S. DEPARTMENT OF THE INTERIOR
U.S. GEOLOGICAL SURVEY**

**LEVEL II BRIDGE SCOUR ANALYSIS FOR STRUCTURE 124000900600
ON ROUTE SC 9, CROSSING THE SANDY RIVER IN CHESTER
COUNTY, SOUTH CAROLINA**

By Andy W. Caldwell and J. Mike Sullivan

**Prepared in cooperation with the
SOUTH CAROLINA DEPARTMENT
OF TRANSPORTATION**



Columbia, South Carolina

1995



UNIT ABBREVIATIONS

cubic foot per second	ft ³ /s
feet per second	ft/s
foot	ft
mile	mi
millimeter	mm
square foot	ft ²
square mile	mi ²

OTHER ABBREVIATIONS

downstream	D/S
upstream	U/S
flood plain	f/p
Water-Surface Profile computation model	WSPRO
median diameter of bed material	D ₅₀
South Carolina Department of Transportation	SCDOT

In this report, the words "right" and "left" refer to directions that would be reported by an observer facing downstream.

Sea level: In this report, "sea level" refers to the National Geodetic Vertical Datum of 1929-- a geodetic datum derived from a general adjustment of the first-order level nets of the United States and Canada, formerly called Sea Level Datum of 1929.

**Level II bridge scour analysis
for structure 124000900600 on Route SC 9,
crossing the Sandy River in Chester County, South Carolina**

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This report provides the results of the detailed Level II analysis of scour potential at structure 124000900600 on Route SC 9, crossing the Sandy River in Chester County, South Carolina (figure 1 in pocket; figures 5-8). The site is located in the Piedmont physiographic province near the town of Chester in the central part of Chester County. The drainage area for the site is 16.7 mi², and is a predominantly rural drainage basin with little development in recent years. In the vicinity of the study site, the land is covered by moderate to dense hardwoods upstream and grassy fields turning into moderate hardwoods approximately 400 ft downstream.

In the study area, the Sandy River has a meandering channel with a slope of approximately 0.0013 ft/ft (6.9 ft/mi), an average channel top width of 80 ft and an average channel depth of 9 ft. The predominant channel bed material is sand (D₅₀ is 0.28 mm) and the predominant bank material is a coarser sand (D₅₀ is 0.46 mm). In general, the banks have moderate woody vegetative cover with some bank failure noted at the time of the Level I site visit on July 17, 1990.

The Route SC 9 crossing of the Sandy River is a 180-ft-long, two-lane bridge consisting of two 40-ft and two 50-ft concrete spans, supported by steel and concrete bents with spillthrough abutments. The abutments are protected by riprap. In this report, the words "right" and "left" refer to directions that would be reported by an observer facing downstream. Additional details describing conditions at the site are included in the Scour Report Summary.

Scour depths were computed using engineering judgement and the general guidelines described in Hydraulic Engineering Circular 18 (Richardson and others, 1993) and the Transportation Research Board Draft Paper, "Evaluating scour at bridges using WSPRO" (Arneson and others, 1992). Scour depths were calculated assuming an infinite depth of erosive material and a homogeneous particle-size distribution. The results of the scour analysis are presented in tables 1 and 2 and a graph of the scour depths is shown on figure 2.

Pile penetration depths were obtained from the SCDOT bridge plans (docket number 12.388). Pile tip exposure of 9.4 ft and 16.3 ft occurs for the 100- and 500-year discharges, respectively. This exposure occurs at bent 3.

Table 1. --Remaining pile/footing penetration at piers/bents for the 100- and 500-year discharges at structure 124000900600 on Route SC 9, crossing the Sandy River in Chester County, South Carolina.

Pier/bent number	Station from left end of bridge (feet)	Pile tip/ ³ footing elevation, SCDOT datum (feet)	Pile tip/ footing elevation, USGS datum (feet)	Ground elevation at pier/bent, USGS datum (feet)	Total ⁴ scour depth (feet)	Elevation of scour, USGS datum (feet)	Remaining ⁵ pile/footing penetration (feet)
100-year discharge is 4,400 cubic feet per second							
4	40	403.2	402.5	416.1	0.0	416.1	13.6
3	90	398.6	397.9	410.6	22.1	388.5	-9.4
2	140	395.5	394.8	409.8	22.1	387.7	-7.1
500-year discharge is 6,600 cubic feet per second							
4	40	403.2	402.5	416.1	6.5	409.6	7.1
3	90	398.6	397.9	410.6	29.0	381.6	-16.3
2	140	395.5	394.8	409.8	29.0	380.8	-14.0

¹ Pier/bent number corresponds to the South Carolina Department of Transportation (SCDOT) bridge plans (docket number 12.388).

² Stations are determined from left to right looking downstream.

³ Pile tip/footing elevations obtained from the SCDOT bridge plans. The maximum elevation at each pier/bent is used.

⁴ Total scour depth is the sum of the contraction and pier/bent scour depths.

⁵ A negative number signifies undermining of pile tip/footing.

Table 2. --Cumulative scour depths at piers/bents for the 100- and 500-year discharges at structure 124000900600 on Route SC 9, crossing the Sandy River in Chester County, South Carolina.

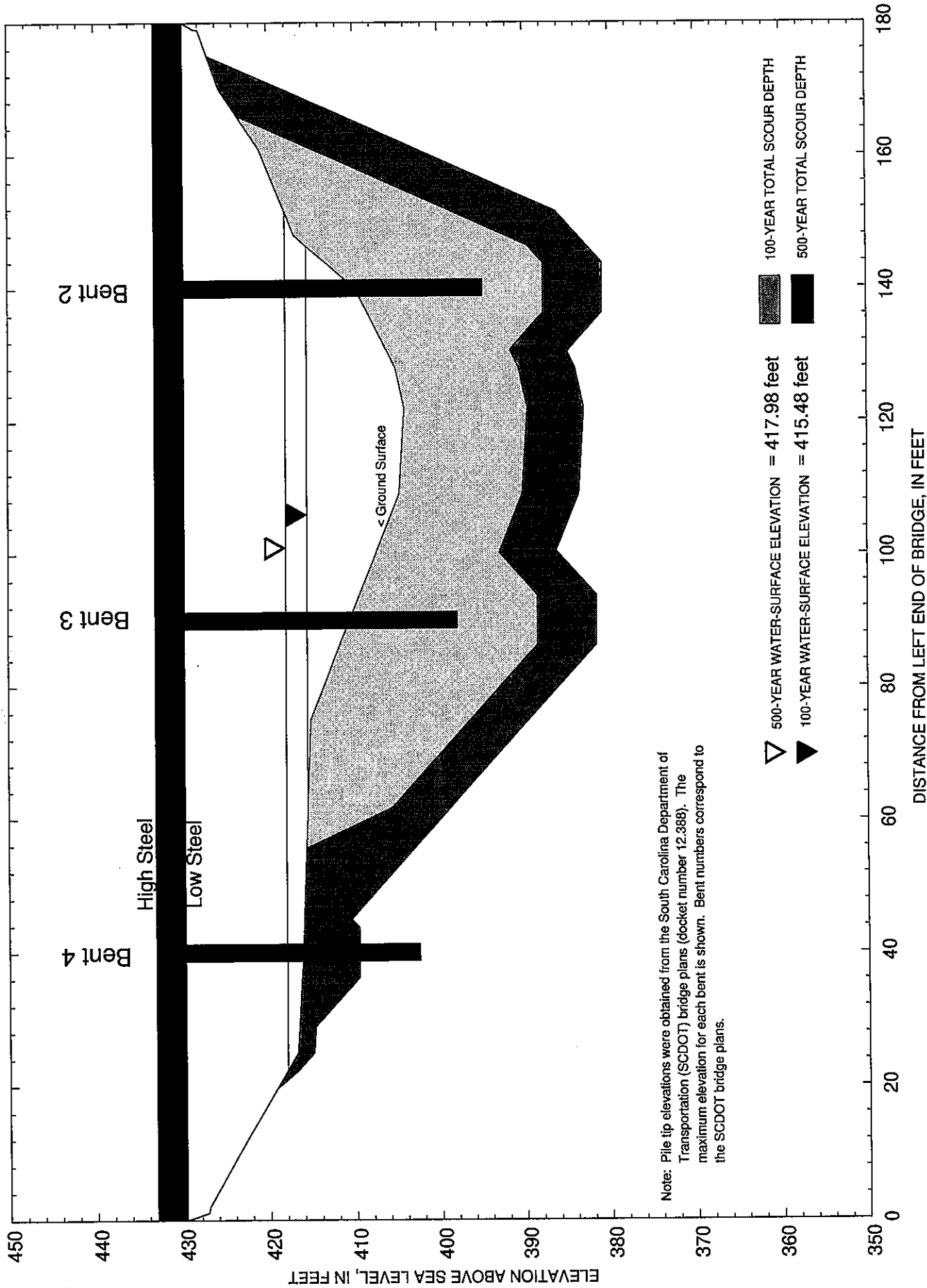
Pier/bent ¹ number	Station from ² left end of bridge (feet)	Contraction scour depth (feet)	Pier/bent scour depth without debris (feet)	Total ³ scour depth without debris (feet)
100-year discharge is 4,400 cubic feet per second				
4	40	0.0	0.0	0.0
3	90	14.5	7.6	22.1
2	140	14.5	7.6	22.1
500-year discharge is 6,600 cubic feet per second				
4	40	1.9	4.6	6.5
3	90	21.1	7.9	29.0
2	140	21.1	7.9	29.0

¹ Pier/bent number corresponds to the South Carolina Department of Transportation bridge plans (docket number 12.388).

² Stations are determined from left to right looking downstream.

³ Total scour depth is the sum of the contraction and pier/bent scour depths.

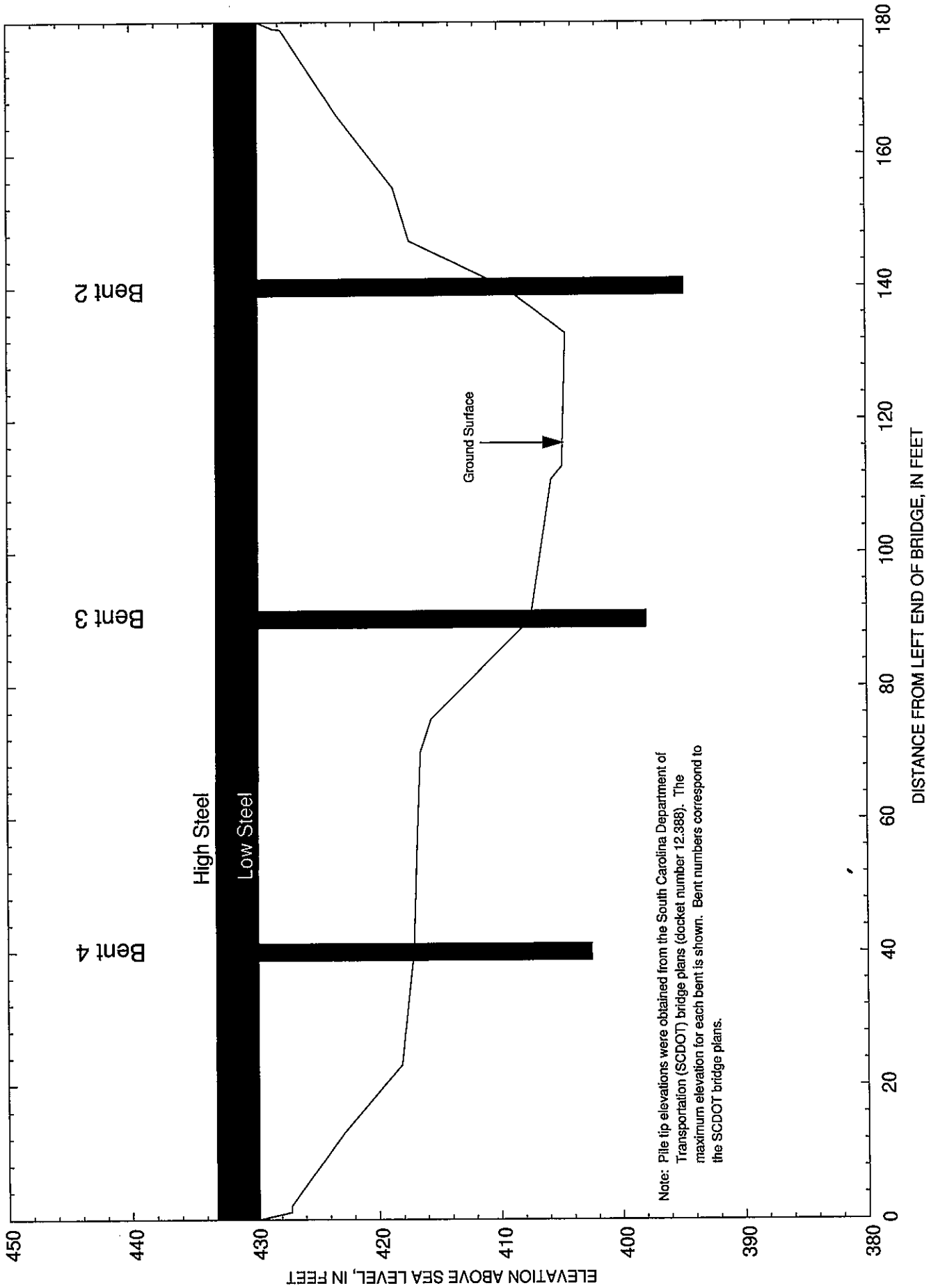
NOTE: The pier and contraction scour equations used in this scour analysis were those recommended in Hydraulic Engineering Circular 18 (Richardson and others, 1993). Scour depths were calculated assuming an infinite depth of erosive material and a homogeneous particle-size distribution.



Note: Pile tip elevations were obtained from the South Carolina Department of Transportation (SCDOT) bridge plans (docket number 12.388). The maximum elevation for each bent is shown. Bent numbers correspond to the SCDOT bridge plans.

Figure 2.-- Total scour depths for the 100- and 500-year discharges at the downstream face of structure 124000900600 on Route SC 9, crossing the Sandy River in Chester County, South Carolina.





Note: Pile tip elevations were obtained from the South Carolina Department of Transportation (SCDOT) bridge plans (docket number 12.388). The maximum elevation for each bent is shown. Bent numbers correspond to the SCDOT bridge plans.

Figure 3.--The upstream face of structure 12400900600 on Route SC 9, crossing the Sandy River in Chester County, South Carolina.



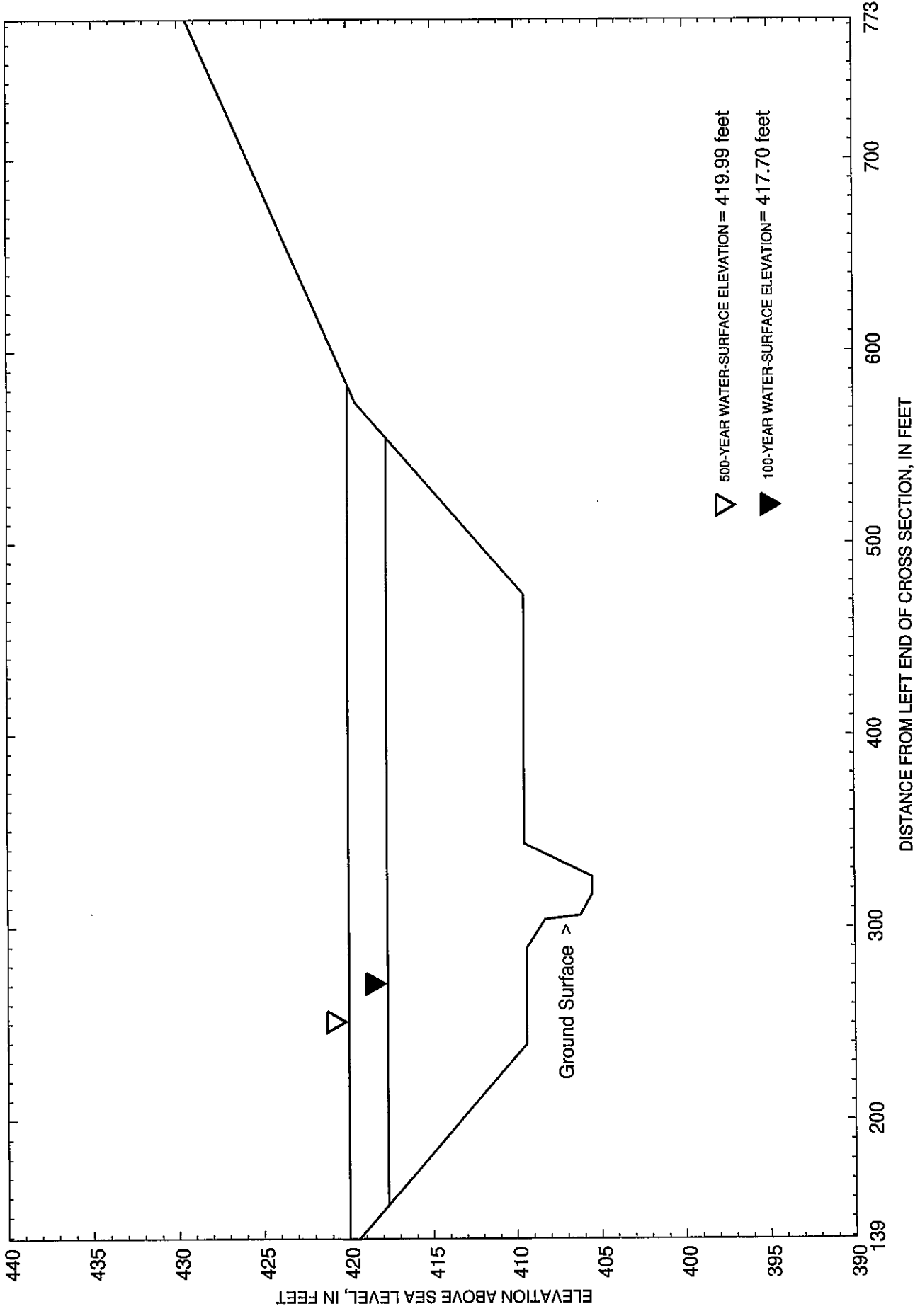


Figure 4.--Approach cross section at structure 124000900600 on Route SC 9, crossing the Sandy River in Chester County, South Carolina.





Figure 5.--Structure 124000900600 on Route SC 9, crossing the Sandy River in Chester County, South Carolina as viewed from the upstream channel (July 17, 1990).



Figure 6.--Upstream channel as viewed from the approach cross section of structure 124000900600 on Route SC 9, crossing the Sandy River in Chester County, South Carolina (September 7, 1993).





Figure 7.--Structure 124000900600 on Route SC 9, crossing the Sandy River in Chester County, South Carolina as viewed from the downstream channel (July 17, 1990).



Figure 8.--Downstream channel as viewed from structure 124000900600 on Route SC 9, crossing the Sandy River in Chester County, South Carolina (July 17, 1990).



SELECTED REFERENCES

- Arcement, G.J., Jr., and Schneider, V.R., 1989, Guide for selecting Manning's roughness coefficients for natural channels and flood plains: U.S. Geological Survey Water-Supply Paper 2339, 38 p.
- Arneson, L. A., Shearman, J. O., Jones, J. S., 1992, Evaluating scour at bridges using WSPRO: Transportation Research Board Draft Paper, 40 p.
- Bohman, L. R., 1990, Determination of flood hydrographs for streams in South Carolina: Volume 1. Simulation of flood hydrographs for rural watersheds in South Carolina: U.S. Geological Survey Water-Resources Investigations Report 89-4087, 53 p.
- Bohman, L. R., 1992, Determination of flood hydrographs for streams in South Carolina: Volume 2. Estimation of peak-discharge frequency, runoff volumes, and flood hydrographs for urban watersheds: U.S. Geological Survey Water-Resources Investigations Report 92-4040, 79 p.
- Froehlich, D. C., 1989, Local scour at bridge abutments in Ports, M. A., ed., Hydraulic Engineering--Proceedings of the 1989 National Conference on Hydraulic Engineering: New York, American Society of Civil Engineers, p. 13-18.
- Guimaraes, W. B., and Bohman, L. R., 1991, Techniques for estimating magnitude and frequency of floods in South Carolina, 1988: U.S. Geological Survey Water-Resources Investigation Report, 91-4157, 174 p.
- Gunter, H.E., Mason, R.R., and Stamey, T.C., 1987, Magnitude and frequency of floods in rural and urban basins in North Carolina: U.S. Geological Survey Water-Resources Investigations Report, 87-4096, 54 p.
- Laursen, E. M., 1960, Scour at bridge crossings: Journal of the Hydraulics Division, American Society of Civil Engineers, v. 86, no. HY2, p. 39-53.
- Laursen, E. M., 1963, An analysis of relief bridge scour: Journal of the Hydraulics Division, American Society of Civil Engineers, v. 89, no. HY3, p. 93-118.
- Richardson, E. V., Harrison, L. J., Richardson, J. R., and Davis, S. R., 1993, Evaluating scour at bridges: Federal Highway Administration Hydraulic Engineering Circular No. 18, Publication FHWA-IP-90-017, 131 p.
- Richardson, E. V., Simons, D. B., and Julien, P. Y., 1990, Highways in the river environment: Federal Highway Administration Publication FHWA-HI-90-016.
- Richardson, E. V., Simons, D. B., Karaki, S., Mahmood, K., and Stevens, M. A., 1975, Highways in the river environment: hydraulic and environmental design considerations: Federal Highway Administration.
- Shearman, J. O., 1990, User's manual for WSPRO--a computer model for water surface profile computations: Federal Highway Administration Publication FHWA-IP-89-027, 187 p.
- Shearman, J. O., Kirby, W. H., Schneider, V. R., and Flippo, H. N., 1986, Bridge waterways analysis model; research report: Federal Highway Administration Publication FHWA-RD-86-108, 112 p.
- U.S. Geological Survey, Interagency Advisory Committee on Water Data, 1982, Guidelines for determining flood flow frequency, Bulletin 17B of the Hydrology Subcommittee, 190 p.



SCOUR REPORT SUMMARY

Structure Number 124000900600 **Stream** Sandy River
County Chester **Road** SC 9 **District** 4

Description of Bridge

Bridge length 180 ft **Bridge width** 35 ft **Max span length** 50 ft
Alignment of bridge to road (on curve or straight) Straight
Abutment type Spillthrough **Embankment type** Sloping
Riprap on abutment? Yes **Date of inspection** 7-17-1990
Description of riprap Both abutments are protected by 6- to 16-inch granite riprap.

Brief description of piers/pile bents Three interior bents support the bridge. The existing structure is supported by two 2.8-ft by 2.5-ft concrete columns and the widened part of the structure is supported by tower bents on the U/S and D/S ends consisting of two 0.8-ft-square steel H-piles.

Is bridge skewed to flood plain according to USGS topo map? Yes **Angle** 15

Is bridge located on a bend in channel? No *If so, describe (mild, moderate, severe)*

Debris accumulation on bridge at time of Level I or Level II site visit:

	<i>Date of inspection</i>	<i>Percent of channel blocked horizontally</i>	<i>Percent of channel blocked vertically</i>
<i>Level I</i>	<u>7-17-1990</u>	<u>0</u>	<u>0</u>
<i>Level II</i>	<u>8-31-1993</u>	<u>0</u>	<u>0</u>

Potential for debris Low: Chester Reservoir is located approximately 1,100 ft upstream of the Route SC 9 crossing.

Describe any features near or at the bridge that may affect flow (include observation date).
The Chester Reservoir is located approximately 1,100 ft upstream of the Route SC 9 crossing.

Description of Flood Plain

General topography Typical Piedmont topography with rolling hills

Flood-plain conditions at bridge site: downstream (D/S), upstream (U/S)

Date of inspection 8-31-1993

D/S left: Two- to 3-ft high grass

D/S right: Two- to 3-ft high grass

U/S left: Moderate hardwoods with moderate undergrowth

U/S right: Sparse hardwoods with thick undergrowth

Description of Channel

Average top width 80 ft *Average depth* 9.0 ft

Predominant bed material sand *Bank material* sand

Stream type (straight, meandering, braided, swampy, channelized) Meandering

Vegetative cover on channel banks near bridge: Date of inspection 8-31-1993

D/S left: Thin woody vegetation

D/S right: Thin woody vegetation

U/S left: Moderate woody vegetation

U/S right: Moderate woody vegetation

Do banks appear stable? Yes* *If not, describe location and type of instability and date of observation.* * Some bank failure was noted on the upstream left and downstream right bank at the time of the Level I site visit on 7-17-1990.

Describe any obstructions in channel and date of observation. None observed.

Description of data and assumptions used in developing WSPRO model.

The Sandy River has a relatively uniform flood plain width in the study area, with no downstream natural or man-made contractions of flow that cause significant backwater at the Route SC 9 crossing. Therefore, it was assumed that slope-conveyance methodology would be adequate for estimating the starting water-surface elevation for the water-surface profile computations.

For this study, the WSPRO model requires, as a minimum, an exit cross section one bridge width downstream of the bridge, a full-valley cross section at the downstream bridge face, the bridge cross section, and an approach cross section one bridge width upstream of the bridge. Cross sections at the upstream and downstream faces of the bridge were directly surveyed and the more constricted (downstream) bridge face was used in the WSPRO model. The section reference distance (SRD) at the downstream face of the bridge was set to zero. An exit cross section was surveyed approximately 372 ft downstream of the downstream bridge face and an approach channel cross section was surveyed approximately 199 ft upstream of the upstream bridge face. The approach cross section was synthesized by the slope of the contours on the USGS topographic map. These cross sections were shifted by the channel slope to the appropriate SRD to represent the exit, full-valley, and approach cross sections required by the WSPRO model. In addition, the exit cross section was shifted by the channel slope to SRD -400 and -390 to represent cross sections EXITA and EXITB. Cross section EXITA is the starting cross section which represents the wooded flood plain. Cross section EXITB is a transition cross section to show the change from a wooded flood plain to a grassy flood plain.

Bridge Hydraulics

Average embankment elevation 431.0 *ft*

Average low steel elevation 429.7 *ft*

100-year discharge 4,400 *ft³/s*

Water-surface elevation at D/S bridge face 415.48 *ft*

Area of flow at D/S bridge face 515 *ft²*

Average velocity in bridge opening 8.55 *ft/s*

Maximum WSPRO tube velocity at bridge 10.29 *ft/s*

Water-surface elevation at Approach section with bridge 417.70 *ft*

Water-surface elevation at Approach section without bridge 416.22 *ft*

Amount of backwater caused by bridge 1.48 *ft*

500-year discharge 6,600 *ft³/s*

Water-surface elevation at D/S bridge face 417.98 *ft*

Area of flow at D/S bridge face 803 *ft²*

Average velocity in bridge opening 8.22 *ft/s*

Maximum WSPRO tube velocity at bridge 10.71 *ft/s*

Water-surface elevation at Approach section with bridge 419.99 *ft*

Water-surface elevation at Approach section without bridge 418.68 *ft*

Amount of backwater caused by bridge 1.31 *ft*

Scour

Describe any special assumptions or considerations made in bridge scour analysis.

Scour depths were computed using engineering judgement and the general guidelines described in Hydraulic Engineering Circular 18 (Richardson and others, 1993) and the Transportation Research Board Draft Paper, "Evaluating scour at bridges using WSPRO" (Arneson and others, 1992). Scour depths were calculated assuming an infinite depth of erosive material and a homogeneous particle-size distribution. The results of the scour analysis are presented in tables 1 and 2 and a graph of the scour depths is shown on figure 2.

The local pier scour was determined using the Colorado State University pier scour equation (Richardson and others, 1993). Bent 4 is located on the left overbank and was analyzed using the maximum left overbank WSPRO tube velocity and the depth of flow at the bent. Bents 2 and 3 are located in the channel and were analyzed using 90 percent of the maximum WSPRO tube velocity and the maximum depth within the channel at the bridge. The maximum depth within the channel was used to account for possible changes in the thalweg during a flood. The bridge is skewed approximately 15 degrees to the channel. However, because the columns are spaced far apart, they function as if there were no skew.

The left overbank at the bridge was analyzed for contraction scour using Laursen's clear-water contraction scour equation (Richardson and others, 1993).

Chester Reservoir is located approximately 1,100 ft upstream of the Route SC 9 bridge. Therefore, sediment transport is likely to be minimal and it was decided that clear-water scour would best represent the contraction scour processes at the bridge; consequently, the potential contraction scour was determined using Laursen's clear-water contraction scour equation (Richardson and others, 1993). In addition, it was decided to neglect subtracting the pier widths and to use the higher flood plain D_{50} to try to obtain more reasonable contraction scour results. However, the clear-water contraction scour results, 14.5 ft and 21.1 ft for the 100- and 500-year discharges, respectively, appear excessive and therefore, engineering judgement should be exercised when interpreting these results.

No abutment scour computations were made because the abutments are protected by riprap.

WSPRO OUTPUT

WSPRO
V042094

FEDERAL HIGHWAY ADMINISTRATION - U. S. GEOLOGICAL SURVEY
MODEL FOR WATER-SURFACE PROFILE COMPUTATIONS

Structure #124000900600 (180 ft. bridge)
Sandy River at SC 9 file: sandy.sc9
Chester County, South Carolina AWC September 1994
*** RUN DATE & TIME: 03-15-95 15:25

CROSS-SECTION PROPERTIES: ISEQ = 5; SECID = BRIDG; SRD = 0.

WSEL	SA#	AREA	K	TOPW	WETP	ALPH	LEW	REW	QCR
	1	3	25	13	13				6
	2	512	54391	71	76				7782
415.48		515	54416	85	89	1.01	62	146	7167

VELOCITY DISTRIBUTION: ISEQ = 5; SECID = BRIDG; SRD = 0.

	WSEL	LEW	REW	AREA	K	Q	VEL
	415.88	47.7	146.9	551.4	59414.	4400.	7.98
X STA.		47.7	90.4	96.1	100.1	103.2	105.9
A(I)		58.4	35.9	31.0	27.7	26.0	
V(I)		3.77	6.12	7.10	7.93	8.45	
X STA.		105.9	108.2	110.3	112.3	114.2	116.1
A(I)		24.4	23.3	22.4	21.6	21.8	
V(I)		9.02	9.43	9.81	10.19	10.11	
X STA.		116.1	117.9	119.8	121.6	123.5	125.4
A(I)		21.6	21.7	21.4	21.6	22.5	
V(I)		10.20	10.13	10.29	10.17	9.77	
X STA.		125.4	127.5	129.9	132.7	136.5	146.9
A(I)		22.7	24.9	26.5	31.4	44.5	
V(I)		9.69	8.82	8.30	7.00	4.95	

WSPRO OUTPUT --Continued

WSPRO
V042094

FEDERAL HIGHWAY ADMINISTRATION - U. S. GEOLOGICAL SURVEY
MODEL FOR WATER-SURFACE PROFILE COMPUTATIONS

Structure #124000900600 (180 ft. bridge)
Sandy River at SC 9 file: sandy.sc9
Chester County, South Carolina AWC September 1994

*** RUN DATE & TIME: 03-15-95 15:25

CROSS-SECTION PROPERTIES: ISEQ = 6; SECID = APPR ; SRD = 215.

WSEL	SA#	AREA	K	TOPW	WETP	ALPH	LEW	REW	QCR
	1	763	22682	133	134				10355
	2	569	72722	54	55				10469
	3	1407	52803	212	213				20566
417.70		2739	148207	399	402	2.96	156	555	23657

VELOCITY DISTRIBUTION: ISEQ = 6; SECID = APPR ; SRD = 215.

	WSEL	LEW	REW	AREA	K	Q	VEL
	417.70	155.8	555.2	2738.8	148207.	4400.	1.61
X STA.	155.8	235.7	262.6	287.5	296.3	303.1	
A(I)	319.5	223.4	207.1	75.1	61.9		
V(I)	0.69	0.98	1.06	2.93	3.55		
X STA.	303.1	308.9	313.4	317.7	322.0	326.3	
A(I)	63.1	53.2	52.7	52.5	52.5		
V(I)	3.49	4.13	4.18	4.19	4.19		
X STA.	326.3	331.0	336.7	345.7	366.9	388.2	
A(I)	54.9	59.2	78.1	175.0	175.1		
V(I)	4.01	3.71	2.82	1.26	1.26		
X STA.	388.2	409.7	431.5	454.6	478.3	555.2	
A(I)	176.9	179.2	189.6	194.0	295.8		
V(I)	1.24	1.23	1.16	1.13	0.74		

WSPRO OUTPUT --Continued

WSPRO
V042094

FEDERAL HIGHWAY ADMINISTRATION - U. S. GEOLOGICAL SURVEY
MODEL FOR WATER-SURFACE PROFILE COMPUTATIONS

Structure #124000900600 (180 ft. bridge)
Sandy River at SC 9 file: sandy.sc9
Chester County, South Carolina AWC September 1994

*** RUN DATE & TIME: 03-15-95 15:25

CROSS-SECTION PROPERTIES: ISEQ = 5; SECID = BRIDG; SRD = 0.

WSEL	SA#	AREA	K	TOPW	WETP	ALPH	LEW	REW	QCR
	1	108	5163	53	53				876
	2	693	88503	73	78				12128
	3	2	36	4	4				8
417.98		803	93703	129	135	1.14	22	152	10647

VELOCITY DISTRIBUTION: ISEQ = 5; SECID = BRIDG; SRD = 0.

	WSEL	LEW	REW	AREA	K	Q	VEL
	418.26	21.8	152.4	839.5	99306.	6600.	7.86
X STA.	21.8	67.7	84.9	91.9	96.7	100.5	
A(I)	100.3	68.5	50.0	42.8	39.8		
V(I)	3.29	4.82	6.60	7.71	8.30		
X STA.	100.5	103.7	106.5	109.1	111.4	113.7	
A(I)	36.4	34.6	33.9	31.2	31.5		
V(I)	9.07	9.53	9.72	10.56	10.48		
X STA.	113.7	115.9	118.2	120.4	122.6	124.9	
A(I)	31.1	31.3	30.8	31.5	32.3		
V(I)	10.62	10.54	10.71	10.48	10.22		
X STA.	124.9	127.4	130.1	133.4	137.6	152.4	
A(I)	33.2	34.6	38.8	43.0	63.9		
V(I)	9.94	9.53	8.51	7.67	5.17		

WSPRO OUTPUT --Continued

WSPRO
V042094

FEDERAL HIGHWAY ADMINISTRATION - U. S. GEOLOGICAL SURVEY
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Structure #124000900600 (180 ft. bridge)
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Chester County, South Carolina AWC September 1994
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CROSS-SECTION PROPERTIES: ISEQ = 6; SECID = APPR ; SRD = 215.

WSEL	SA#	AREA	K	TOPW	WETP	ALPH	LEW	REW	QCR
	1	1092	38022	150	151				16724
	2	692	100950	54	55				14064
	3	1921	81627	240	241				30822
419.99		3705	220600	444	447	2.99	139	583	35098

VELOCITY DISTRIBUTION: ISEQ = 6; SECID = APPR ; SRD = 215.

	WSEL	LEW	REW	AREA	K	Q	VEL
	419.99	139.0	583.3	3705.5	220600.	6600.	1.78
X STA.	139.0	224.1	253.0	278.4	293.3	300.6	
A(I)		414.4	295.5	269.4	159.4	82.1	
V(I)		0.80	1.12	1.23	2.07	4.02	
X STA.	300.6	307.4	312.4	317.2	322.0	326.8	
A(I)		83.7	70.4	69.6	69.5	69.4	
V(I)		3.94	4.69	4.74	4.75	4.76	
X STA.	326.8	332.1	338.4	351.2	372.7	394.3	
A(I)		72.1	78.3	136.5	225.9	227.1	
V(I)		4.58	4.21	2.42	1.46	1.45	
X STA.	394.3	415.8	438.0	461.7	488.1	583.3	
A(I)		226.6	232.8	249.2	266.8	406.6	
V(I)		1.46	1.42	1.32	1.24	0.81	

WSPRO OUTPUT --Continued

WSPRO
V042094

FEDERAL HIGHWAY ADMINISTRATION - U. S. GEOLOGICAL SURVEY
MODEL FOR WATER-SURFACE PROFILE COMPUTATIONS

Structure #124000900600 (180 ft. bridge)
Sandy River at SC 9 file: sandy.sc9
Chester County, South Carolina AWC September 1994
*** RUN DATE & TIME: 03-15-95 15:25

XSID:CODE	SRDL	LEW	AREA	VHD	HF	EGL	CRWS	Q	WSEL
SRD	FLEN	REW	K	ALPH	HO	ERR	FR#	VEL	
EXITA:XS	*****	42	1197	0.30	****	415.65	407.68	4400	415.35
-399	*****	274	121966	1.43	****	*****	0.34	3.68	
EXITB:XS	10	42	1210	0.24	0.01	415.66	*****	4400	415.42
-389	10	276	132480	1.18	0.00	0.00	0.31	3.64	
EXIT :XS	210	42	1202	0.25	0.23	415.90	*****	4400	415.66
-179	210	275	131547	1.18	0.00	0.01	0.31	3.66	
FULV :FV	180	42	1196	0.25	0.20	416.11	*****	4400	415.87
0	180	274	130869	1.18	0.00	0.01	0.31	3.68	
<<<<<THE ABOVE RESULTS REFLECT "NORMAL" (UNCONSTRICTED) FLOW>>>>>									
APPR :AS	215	171	2168	0.19	0.29	416.41	*****	4400	416.22
215	215	540	108811	2.95	0.00	0.00	0.25	2.03	
<<<<<THE ABOVE RESULTS REFLECT "NORMAL" (UNCONSTRICTED) FLOW>>>>>									

<<<<<RESULTS REFLECTING THE CONSTRICTED FLOW FOLLOW>>>>>

XSID:CODE	SRDL	LEW	AREA	VHD	HF	EGL	CRWS	Q	WSEL
SRD	FLEN	REW	K	ALPH	HO	ERR	FR#	VEL	
BRIDG:BR	180	62	515	1.47	0.49	416.95	412.83	4400	415.48
0	180	146	54429	1.30	0.56	0.00	0.70	8.55	
TYPE PPCD FLOW C P/A LSEL BLEN XLAB XRAB									
3. 1. 1. 0.878 0.051 429.70 ***** ***** *****									
XSID:CODE	SRDL	LEW	AREA	VHD	HF	EGL	CRWS	Q	WSEL
SRD	FLEN	REW	K	ALPH	HO	ERR	FR#	VEL	
APPR :AS	180	156	2739	0.12	0.46	417.82	412.03	4400	417.70
215	193	555	148198	2.96	0.40	0.01	0.19	1.61	
M(G)	M(K)	KQ	XLKQ	XRKQ	OTEL				
0.733	0.440	82905.	271.	356.	417.54				

<<<<<END OF BRIDGE COMPUTATIONS>>>>>

WSPRO OUTPUT --Continued

WSPRO
V042094

FEDERAL HIGHWAY ADMINISTRATION - U. S. GEOLOGICAL SURVEY
MODEL FOR WATER-SURFACE PROFILE COMPUTATIONS

Structure #124000900600 (180 ft. bridge)
Sandy River at SC 9 file: sandy.sc9
Chester County, South Carolina AWC September 1994
*** RUN DATE & TIME: 03-15-95 15:25

XSID:CODE	SRDL	LEW	AREA	VHD	HF	EGL	CRWS	Q	WSEL
SRD	FLEN	REW	K	ALPH	HO	ERR	FR#	VEL	
EXITA:XS	*****	34	1895	0.35	*****	418.30	409.37	6600	417.95
-399	*****	324	182886	1.86	*****	*****	0.33	3.48	
EXITB:XS	10	30	1937	0.20	0.01	418.30	*****	6600	418.10
-389	10	325	233483	1.11	0.00	0.00	0.25	3.41	
EXIT :XS	210	33	1907	0.21	0.17	418.48	*****	6600	418.27
-179	210	324	228539	1.12	0.00	0.00	0.25	3.46	
FULV :FV	180	35	1884	0.21	0.15	418.64	*****	6600	418.43
0	180	324	224687	1.12	0.00	0.00	0.26	3.50	
<<<<<THE ABOVE RESULTS REFLECT "NORMAL" (UNCONSTRICTED) FLOW>>>>>									
APPR :AS	215	146	3138	0.20	0.23	418.88	*****	6600	418.68
215	215	565	177415	2.97	0.00	0.00	0.23	2.10	
<<<<<THE ABOVE RESULTS REFLECT "NORMAL" (UNCONSTRICTED) FLOW>>>>>									

<<<<<RESULTS REFLECTING THE CONSTRICTED FLOW FOLLOW>>>>>

XSID:CODE	SRDL	LEW	AREA	VHD	HF	EGL	CRWS	Q	WSEL																				
SRD	FLEN	REW	K	ALPH	HO	ERR	FR#	VEL																					
BRIDG:BR	180	22	803	1.47	0.37	419.45	414.86	6600	417.98																				
0	180	152	93709	1.40	0.60	0.00	0.69	8.22																					
<table border="0" style="width: 100%;"> <tr> <td style="width: 15%;">TYPE</td> <td style="width: 15%;">PPCD</td> <td style="width: 15%;">FLOW</td> <td style="width: 15%;">C</td> <td style="width: 15%;">P/A</td> <td style="width: 15%;">LSEL</td> <td style="width: 15%;">BLEN</td> <td style="width: 15%;">XLAB</td> <td style="width: 15%;">XRAB</td> <td></td> </tr> <tr> <td>3.</td> <td>1.</td> <td>1.</td> <td>0.844</td> <td>0.054</td> <td>429.70</td> <td>*****</td> <td>*****</td> <td>*****</td> <td></td> </tr> </table>										TYPE	PPCD	FLOW	C	P/A	LSEL	BLEN	XLAB	XRAB		3.	1.	1.	0.844	0.054	429.70	*****	*****	*****	
TYPE	PPCD	FLOW	C	P/A	LSEL	BLEN	XLAB	XRAB																					
3.	1.	1.	0.844	0.054	429.70	*****	*****	*****																					

===140 AT SECID "APPR ": END OF CROSS SECTION EXTENDED VERTICALLY.
WSEL, YLT, YRT = 419.99 419.4 429.5

XSID:CODE	SRDL	LEW	AREA	VHD	HF	EGL	CRWS	Q	WSEL
SRD	FLEN	REW	K	ALPH	HO	ERR	FR#	VEL	
APPR :AS	180	139	3707	0.15	0.41	420.14	412.96	6600	419.99
215	193	583	220721	2.99	0.28	0.01	0.19	1.78	
M(G)	M(K)	KQ	XLKQ	XRKQ	OTEL				
0.686	0.374	137904.	236.	365.	419.83				

<<<<<END OF BRIDGE COMPUTATIONS>>>>>

PIER SCOUR COMPUTATIONS
 FOR
 Sandy River at Str. 124000900600 in Chester Cty., SC
 Q100 AWC 3-16-1995

HYDRAULIC VARIABLES USED IN CSU EQUATION

PIER NUMBER	4	3	2
PIER STATION (FT)	40	90	140
LOCATION OF PIER	lfp	mcl	mcr
Y1: DEPTH (FT)	0.0	11.8	11.8
V1: VEL. (FPS)	0.0	9.3	9.3
a: PIER WIDTH (FT)	2.5	2.5	2.5
L: PIER LENGTH (FT)	7.2	7.2	7.2
PIER SHAPE	1	1	1
ATTACK ANGLE	0	0	0
K1 (SHAPE COEF.)	1.10	1.10	1.10
K2 (ANGLE COEF.)	1.00	1.00	1.00
FROUDE NO.	0.00	0.48	0.48

COMPUTED SCOUR DEPTHS USING CSU EQUATION

SCOUR DEPTH (FT)	0.00	6.87	6.87
MAX SCOUR DEPTH (FT)	0.00	7.56	7.56

"MAX SCOUR DEPTH" includes an additional 10 percent of the computed CSU scour depth as recommended in HEC 18

CONTRACTION SCOUR COMPUTATIONS
 FOR
 Sandy River at Str. 124000900600 in Chester Cty., SC
 Q100 AWC 3-16-1995

MAIN CHANNEL IN BRIDGE OPENING
 CLEAR-WATER CONTRACTION SCOUR COMPUTATIONS

DISCHARGE IN CONTRACTED SECTION (CFS)	=	4400.
WIDTH OF CONTRACTED SECTION (FT)	=	73.0
MEDIAN GRAIN SIZE (FT)	=	0.0019
COMPUTED DEPTH OF CONTRACTED SECTION (FT)	=	25.8
AVERAGE FLOOD PLAIN DEPTH (FT)	=	11.3
DEPTH OF CONTRACTION SCOUR (FT)	=	14.5

PIER SCOUR COMPUTATIONS
 FOR
 Sandy River at Str. 124000900600 in Chester Cty., SC
 Q500 AWC 3-16-1995

HYDRAULIC VARIABLES USED IN CSU EQUATION

PIER NUMBER	4	3	2
PIER STATION (FT)	40	90	140
LOCATION OF PIER	lfp	mcl	mcr
Y1: DEPTH (FT)	2.2	14.2	14.2
V1: VEL. (FPS)	4.8	9.6	9.6
a: PIER WIDTH (FT)	2.5	2.5	2.5
L: PIER LENGTH (FT)	7.2	7.2	7.2
PIER SHAPE	1	1	1
ATTACK ANGLE	0	0	0
K1 (SHAPE COEF.)	1.10	1.10	1.10
K2 (ANGLE COEF.)	1.00	1.00	1.00
FROUDE NO.	0.57	0.45	0.45

COMPUTED SCOUR DEPTHS USING CSU EQUATION

SCOUR DEPTH (FT)	4.14	7.17	7.17
MAX SCOUR DEPTH (FT)	4.55	7.89	7.89

"MAX SCOUR DEPTH" includes an additional 10 percent of the computed CSU scour depth as recommended in HEC 18

CONTRACTION SCOUR COMPUTATIONS
FOR
Sandy River at Str. 124000900600 in Chester Cty., SC
Q500 AWC 3-16-1995

LEFT OVERBANK IN BRIDGE OPENING
CLEAR-WATER CONTRACTION SCOUR COMPUTATIONS

DISCHARGE IN CONTRACTED SECTION (CFS)	=	364.
WIDTH OF CONTRACTED SECTION (FT)	=	50.0
MEDIAN GRAIN SIZE (FT)	=	0.0019
COMPUTED DEPTH OF CONTRACTED SECTION (FT)	=	4.2
AVERAGE FLOOD PLAIN DEPTH (FT)	=	2.3
DEPTH OF CONTRACTION SCOUR (FT)	=	1.9

MAIN CHANNEL IN BRIDGE OPENING
CLEAR-WATER CONTRACTION SCOUR COMPUTATIONS

DISCHARGE IN CONTRACTED SECTION (CFS)	=	6236.
WIDTH OF CONTRACTED SECTION (FT)	=	73.0
MEDIAN GRAIN SIZE (FT)	=	0.0019
COMPUTED DEPTH OF CONTRACTED SECTION (FT)	=	34.8
AVERAGE FLOOD PLAIN DEPTH (FT)	=	13.7
DEPTH OF CONTRACTION SCOUR (FT)	=	21.1







United States Department of the Interior

U.S. GEOLOGICAL SURVEY
Water Resources Division
Stephenson Center, Suite 129
720 Gracern Road
Columbia, SC 29210-7651

March 21, 1995

William H. Hulbert, P.E.
Hydraulic Engineer
South Carolina Department of Transportation
955 Park Street
Columbia, South Carolina 29202

Dear Mr. Hulbert:

We are pleased to transmit to you another report of the Level II Bridge Scour Program titled, "Level II bridge scour analysis for structure 124000900600 on Route SC 9, crossing the Sandy River in Chester County, South Carolina," by Andy W. Caldwell and J. Mike Sullivan. The technical aspects of the report have been reviewed by the South Carolina District Surface-Water Specialist and the editorial aspects of the report have been reviewed and approved by the South Carolina District Hydraulics Section Chief.

If you have any questions concerning this report please contact me (750-6101) or J. Mike Sullivan (750-6165) and we will be glad to assist you.

Sincerely,

Andy W. Caldwell
Civil Engineer

Enclosure





SHEETS

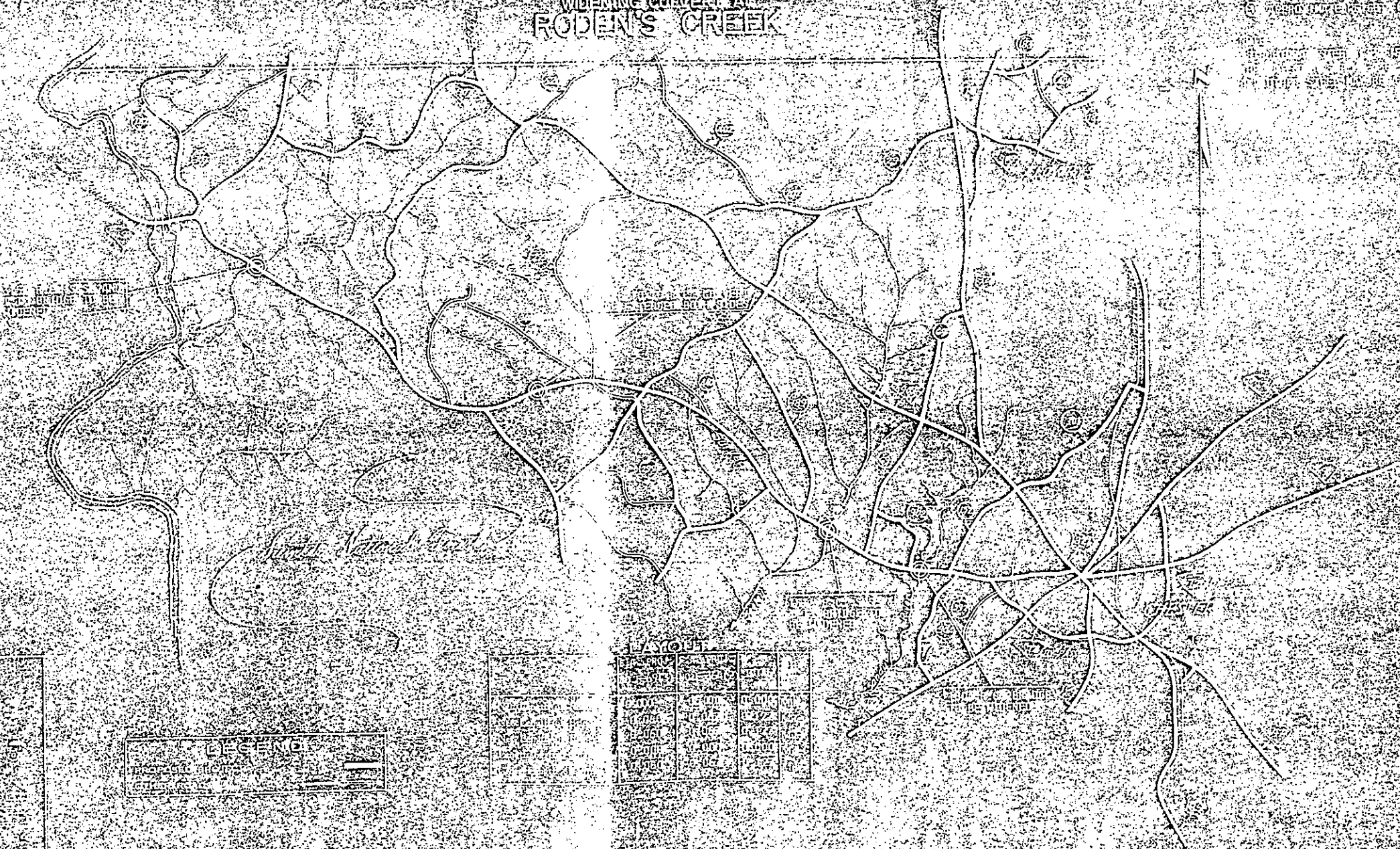
SOUTH CAROLINA
STATE HIGHWAY DEPARTMENT
COLUMBIA

PLAN AND PROFILE OF PROPOSED STATE HIGHWAY

66 6000000 NO. 12 EEE
CHESTER COUNTY
ROUTE NO. 8
E A NO. EEE G
WIDENING BRIDGE OVER
TURKEY CREEK
E A NO. EEE G
WIDENING BRIDGES OVER
SEELEY CREEK & SANDY RIVER G
WIDENING BRIDGE AT
RODEN'S CREEK

SUMMARY OF ESTIMATES	
CONSTRUCTION	1,200,000
LAND ACQUISITION	250,000
LAND CLEARING	150,000
LAND GRADING	100,000
LAND FILLING	50,000
LAND DRAINAGE	20,000
LAND LIGHTING	10,000
LAND SIGNAGE	5,000
LAND FURNISHING	5,000
LAND TOTAL	1,780,000
CONSTRUCTION	1,200,000
LAND TOTAL	2,980,000

CONSTRUCTION	1,200,000
LAND TOTAL	2,980,000
CONSTRUCTION	1,200,000
LAND TOTAL	2,980,000



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W. H. ...

APPROVED

PILE RECORD ON DOCKET NO. 12.388

SANDY RIVER BRIDGE

FED. ROAD DIV. NO.	STATE	COUNTY	DOCKET NO.	ROUTE NO.	SHEET NO.
3	S. C.	Chester	12.388	9	41

WEIGHT OF HAMMER... 3,860 Lbs. TYPE GRAVITY

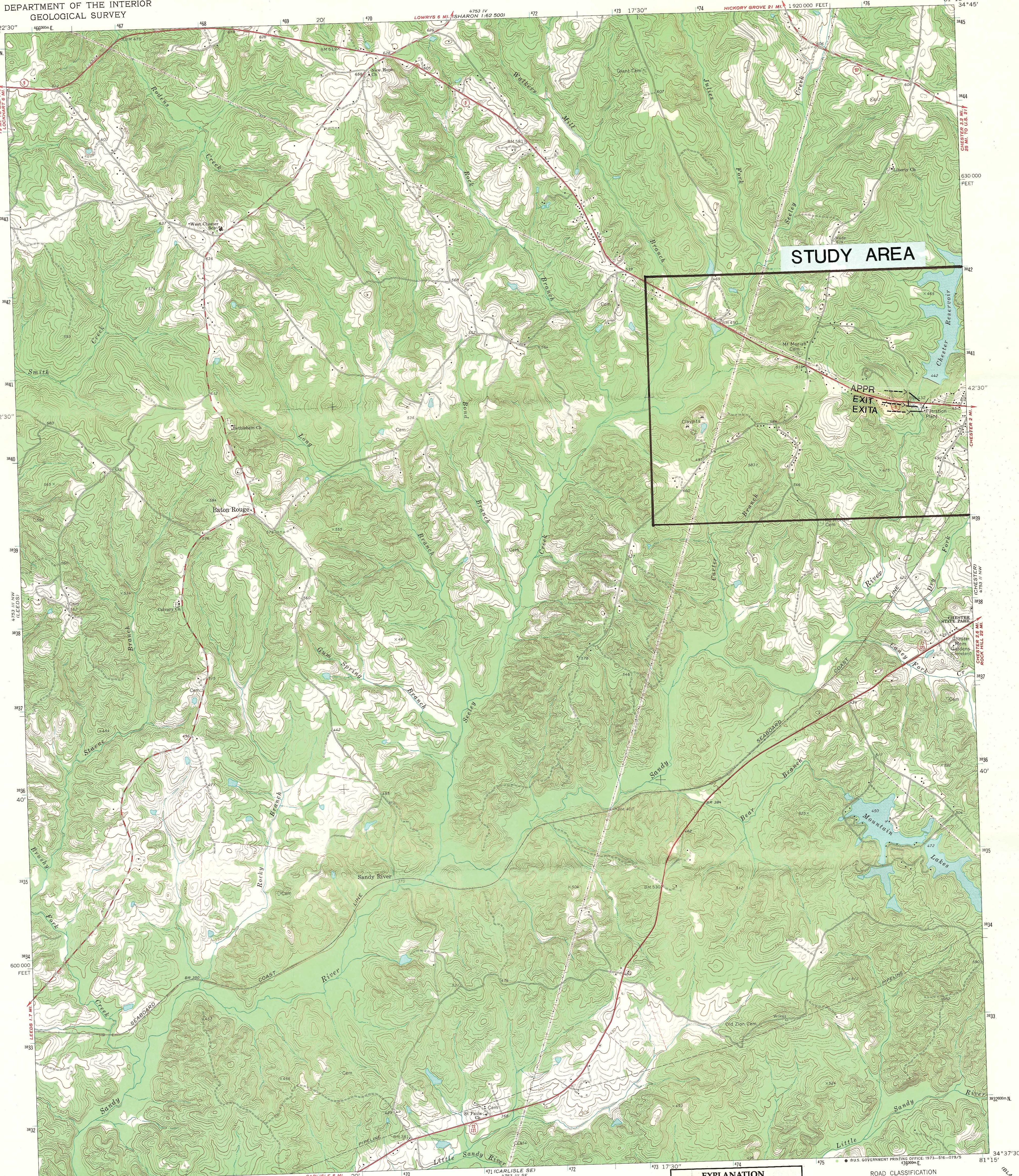
DATE	BENT NO.	FOOTING	PILE NO.	DIAM. AT BUTT.	DIAM. AT TIP	ORIG. LENGTH	BUILD-UP OR SPLICE	TOTAL LENGTH	LENGTH C. O.	NET LENGTH	ELEV. C. O.	ELEV. PILE TIP	ELEV. ORIG. GROUND OR BOTTOM OF FOOTING	PEN. IN GROUND OR BELOW FOOTING	PEN. PER BLOW	FALL OF HAMMER	BEARING VALUE	PAY LENGTH	C.O. @ 40 %	C.O. @ 25 %	DATE	BENT NO.	FOOTING	PILE NO.	DIAM. AT BUTT.	DIAM. AT TIP	ORIG. LENGTH	BUILD-UP OR SPLICE	TOTAL LENGTH	LENGTH C. O.	NET LENGTH	ELEV. C. O.	ELEV. PILE TIP	ELEV. ORIG. GROUND OR BOTTOM OF FOOTING	PEN. IN GROUND OR BELOW FOOTING	PEN. PER BLOW	FALL OF HAMMER	BEARING VALUE	PAY LENGTH	C.O.
																					5-9	1V		1V			35.00		35.00	6.09	28.91	427.27	398.36	425.70	27.34	.583	15'	36.58	28.91	
																					5-9	1V		2V		35.00		35.00	6.07	28.93	427.27	398.34	425.70	27.36	.583	15'	36.58	30.93		
																					5-8	2V		1V		35.00		35.00	4.16	30.84	426.37	395.58	414.94	19.41	.458		39.71	30.84		
																					5-9	2V		2V		35.00		35.00	4.10	30.90	426.37	395.47	413.54	18.07	.708		38.90	30.90		
																					5-9	2V		3V		35.00		35.00	3.60	31.40	426.37	394.97	413.04	18.07	.583		36.58	31.40		
																					5-8	2V		4V		35.00		35.00	3.16	31.84	426.37	394.53	411.14	16.61	.542		37.55	31.84		
																					5-8	3V		1V		35.00		35.00	4.89	30.11	426.37	386.26	408.54	12.28	.500		38.60	30.11		
																					5-9	3V		2V		35.00		35.00	7.21	27.79	426.37	398.58	409.14	10.54	.583		36.58	27.79		
																					5-9	3V		3V		35.00		35.00	5.71	29.29	426.37	397.08	411.64	14.56	.625		35.63	29.29		
																					5-8	3V		4V		35.00		35.00	6.44	28.56	426.37	397.81	412.44	14.63	.458		39.71	28.56		
																					5-8	4V		1V		35.00		35.00	9.64	25.36	426.37	401.01	417.54	16.53	.647		34.73	25.36		
																					5-9	4V		2V		35.00		35.00	11.85	23.15	426.37	403.22	417.54	16.32	.792		32.31	23.15		
																					5-9	4V		3V		35.00		35.00	9.33	25.67	426.37	400.70	416.54	15.84	.458		39.71	25.67		
																					5-8	4V		4V		35.00		35.00	8.29	26.71	426.37	399.66	416.54	16.82	.647		34.73	26.71		
																					5-9	5V		1V		35.00		35.00	10.86	24.14	427.27	403.13	425.70	22.57	.583		36.58	24.14		
																					5-9	5V		2V		35.00		35.00	10.08	24.92	427.27	402.36	425.70	23.34	.542	15'	37.55	24.92		

TOTAL	560.00	111.49	448.51	450.51
GRAND TOTAL	560.00	111.49	448.51	450.51

NOTES CONCERNING ANY UNUSUAL FOUNDATION CONDITIONS		
BENT NO.	FOOTING	REMARKS
1		
2		Due to an engineering error, this pile was cut off 0.90 below cut off elevation. 0.90 was spliced on to bring pile head to proper elevation. This amount was deducted from the original cut-off length and only the remainder was shown above. A 2' allowance was made in the pay length for the splice.

NOTES:
 PAY LENGTH SHOULD INCLUDE ALLOWANCE FOR SPLICING STEEL PILES AND ANY OTHER AUTHORIZED ALLOWANCES.
 NUMBERING PILES: A SKETCH OF BENT OR FOOTING TO BE DRAWN ON THIS SHEET AND PILES TO BE NUMBERED, ALSO FLOW OF STREAM TO BE SHOWN.
 PENETRATION PER BLOW: *W*
 GIVE THIS INFORMATION IN DECIMAL OF AN INCH.

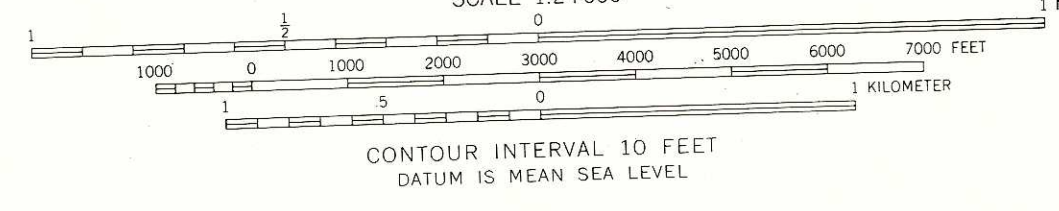
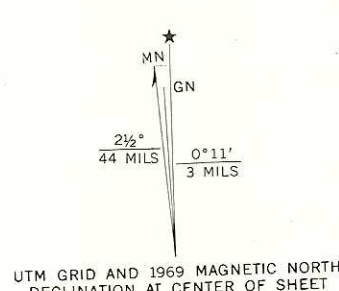
TOTAL



STUDY AREA

APPR
EXIT
EXIT

Mapped, edited, and published by the Geological Survey
Control by USGS and USC&GS
Topography by photogrammetric methods from aerial
photographs taken 1968. Field checked 1969
Polyconic projection. 1927 North American datum
10,000-foot grid based on South Carolina coordinate system, north zone
1000-meter Universal Transverse Mercator grid ticks,
zone 17, shown in blue
Fine red dashed lines indicate selected fence and field lines where
generally visible on aerial photographs. This information is unchecked



EXPLANATION

—	EXIT
—	cross section

ROAD CLASSIFICATION

Primary highway, hard surface	Light-duty road, hard or improved surface
Secondary highway, hard surface	Unimproved road
Interstate Route	U.S. Route
	State Route



BATON ROUGE, S. C.
N3437.5—W8115.7.5
1969

Figure 1.—Topography of study area and location of cross sections used in WSPRO analysis for structure 124000900600 on Route SC 9, crossing the Sandy River in Chester County, South Carolina.