U.S. DEPARTMENT OF THE INTERIOR U.S. GEOLOGICAL SURVEY

LEVEL II BRIDGE SCOUR ANALYSIS FOR STRUCTURE 124007200200 ON ROUTE SC 72, CROSSING THE SANDY RIVER IN CHESTER COUNTY, SOUTH CAROLINA

By J. Mike Sullivan and Andy W. Caldwell

Prepared in cooperation with the SOUTH CAROLINA DEPARTMENT OF TRANSPORTATION



Columbia, South Carolina

UNIT ABBREVIATIONS

cubic foot per second	ft ³ /s
feet per second	ft/s
foot	ft
mile	mi
millimeter	mm
square foot	ft ²
square mile	mi ²

OTHER ABBREVIATIONS

downstream	D/S
upstream	U/S
flood plain	f/p
Water-Surface Profile computation model	WSPRO
median diameter of bed material	D ₅₀
South Carolina Department of Transportation	SCDOT

In this report, the words "right" and "left" refer to directions that would be reported by an observer facing downstream.

Sea level: In this report, "sea level" refers to the National Geodetic Vertical Datum of 1929-- a geodetic datum derived from a general adjustment of the first-order level nets of the United States and Canada, formerly called Sea Level Datum of 1929.

Level II bridge scour analysis for structure 124007200200 on Route SC 72, crossing the Sandy River in Chester County, South Carolina

by J. Mike Sullivan and Andy W. Caldwell

This report provides the results of the detailed Level II analysis of scour potential at structure 124007200200 on Route SC 72, crossing the Sandy River in Chester County, South Carolina (figure 1 in pocket; figures 4-7). The site is located in the Piedmont physiographic province near the town of Leeds in the southwestern part of Chester County. The drainage area for the site is 105 mi², and is a predominantly rural drainage basin with little development in recent years. In the vicinity of the study site, the land is covered by moderate to dense woods consisting of hardwoods and the downstream, right flood plain is a pasture that extends 540 ft downstream of the Route SC 72 crossing.

In the study area, the Sandy River has a meandering channel with a slope of approximately 0.0015 ft/ft (7.9 ft/mi), an average channel top width of 68 ft and an average channel depth of 9.4 ft. The predominant channel bed material is sand (D_{50} is 1.3 mm) and the channel banks consist of a silty sand (D_{50} is 0.88 mm). In general, the banks have moderate to heavy woody vegetative cover and were noted as having some bank failure with exposed tree roots and trees leaning over the channel at the time of the Level I site visit, January 28, 1991, and the Level II site visit, April 20, 1993.

The Route SC 72 crossing of the Sandy River is a 390-ft-long, two-lane bridge consisting of thirteen 30-ft concrete spans, supported by steel and concrete bents with spillthrough abutments. The left and right abutments are protected by riprap. In this report, the words "right" and "left" refer to directions that would be reported by an observer facing downstream. Additional details describing conditions at the site are included in the Scour Report Summary.

Scour depths were computed using engineering judgement and the general guidelines described in Hydraulic Engineering Circular 18 (Richardson and others, 1993) and the Transportation Research Board Draft Paper, "Evaluating scour at bridges using WSPRO" (Arneson and others, 1992). Scour depths were calculated assuming an infinite depth of erosive material and a homogeneous particle-size distribution. The results of the scour analysis are presented in tables 1 through 4 and a graph of the scour depths is shown on figure 2.

Pile penetration depths were obtained from the SCDOT bridge plans. The minimum remaining pile penetration depths for the 100- and 500-year discharges are 12.3 ft and 11.8 ft, respectively. These minimum depths occur at bent 4.

It should be noted that the SCDOT bridge plan borings (file number 12.248) show subsurface rock and gravel deposits that could affect the scour depths shown in this study. For more information, see the SCDOT bridge plans in the pocket at the back of the report.

 Table 1. --Remaining pile/footing penetration at piers/bents for the 100-year discharge at structure 124007200200 on Route SC 72, crossing the Sandy

 River in Chester County, South Carolina

Pier/bent ¹ number	left end of bridge (feet)	footing elevation, SCDOT datum (feet)	footing elevation, USGS datum (feet)	elevation at pier/bent, USGS datum (feet)	Total ⁴ scour depth (feet)	Elevation of scour, USGS datum (feet)	Remaining pile/footing penetration (feet)
		100-year	discharge is 14,	100-year discharge is 14,300 cubic feet per second	er second		
2	30	295.3	295.1	329.5	1.9	327.6	32.5
£	60	296.0	295.8	323.3	3.2	320.1	24.3
4	90	296.9	296.7	314.8	5.8	309.0	12.3
5	120	294.9	294.7	315.4	5.8	309.6	14.9
9	150	292.0	291.8	322.9	3.2	319.7	27.9
7	180	294.4	294.2	323.5	5.9	317.6	23.4
80	210	295.3	295.1	323.8	5.9	317.9	22.8
6	240	297.3	297.1	324.3	5.8	318.5	21.4
10	270	300.8	300.6	323.8	5.9	317.9	17.3
11	300	301.6	301.4	324.5	5.8	318.7	17.3
12	330	300.7	300.5	325.4	5.8	319.6	19.1
13	360	304.3	304.1	331.6	5.3	326.3	22.2

² Stations are determined from left to right looking downstream.

³ Pile tip/footing elevations obtained from the SCDOT bridge plans. The maximum elevation at each pier/bent is used.

⁴ Total scour depth is the sum of the contraction and pier/bent scour depths.

۰.

NOTE: The SCDOT bridge plan borings (docket number 12.248) show subsurface rock and gravel deposits that could reduce the scour depths shown in the above table. For more information, see the SCDOT plans in report pocket.

Pier/bent ¹ number	left end of bridge (feet)	footing elevation, SCDOT datum (feet)	footing elevation, USGS datum (feet)	elevation at pier/bent, USGS datum (feet)	Total ⁴ scour depth (feet)	Elevation of scour, USGS datum (feet)	Remaining pile/footing penetration (feet)
		500-year	discharge is 21,	500-year discharge is 21,000 cubic feet per second	er second		
2	30	295.3	295.1	329.5	2.2	327.3	32.2
3	60	296.0	295.8	323.3	3.4	319.9	24.1
4	06	296.9	296.7	314.8	6.3	308.5	11.8
5	120	294.9	294.7	315.4	6.3	, 309.1	14.4
6	150	292.0	291.8	322.9	3.4	319.5	27.7
7	180	294.4	294.2	323.5	9.9	313.6	19.4
8	210	295.3	295.1	323.8	9.9	313.9	18.8
6	240	297.3	297.1	324.3	9.9	314.4	17.3
10	270 .	300.8	300.6	323.8	6.6	313.9	13.3
11	300	301.6	301.4	324.5	6.6	314.6	13.2
12	330	300.7	300.5	325.4	9.8	315.6	15.1
13	360	304.3	304.1	331,6	9.5	322.1	18.0

.

NOTE: The SCDOT bridge plan borings (docket number 12.248) show subsurface rock and gravel deposits that could reduce the scour depths shown in the above table. For more information, see the SCDOT plans in report pocket.

Pier/bent ¹ number	Station from ² left end of bridge (feet)	Contraction scour depth (feet)	Pier/bent scour depth without debris (feet)	Total ³ scour depth without debris (feet)
	100-year dischar	ge is 14,300 cubi	ic feet per second	
2	30	0.0	1.9	1.9
3	60	0.0 ⁴	3.2	3.2
4	90	0.04	5.8	5.8
5	120	0.0 ⁴	5.8	5.8
6	150	0.04	3.2	3.2
7	180	3.3	2.6	5.9
8	210	3.3	2.6	5.9
9	240	3.3	2.5	5.8
10	270	3.3	2.6	5.9
11	300	3.3	2.5	5.8
12	330	3.3	2.5	5.8
13	360	3.3	2.0	5.3

Table 3 Cumulative scour depths at piers/bents for the 100-year discharge at structure
124007200200 on Route SC 72, crossing the Sandy River in Chester County, South Carolina

¹ Pier/bent number corresponds to the South Carolina Department of Transportation (SCDOT) bridge plans.

² Stations are determined from left to right looking downstream.

³ Total scour depth is the sum of the contraction and pier/bent scour depths.

⁴ The calculated contraction scour is a negative value, but was set equal to zero to reflect a more reasonable estimate of scour during peak flood conditions.

NOTE: The SCDOT bridge plan borings (docket number 12.248) show subsurface rock and gravel deposits that could reduce the scour depths shown in the above table. For more information, see the SCDOT plans in report pocket.

NOTE: The pier and contraction scour equations used in this scour analysis were those recommended in Hydraulic Engineering Circular 18 (Richardson and others, 1993). Scour depths were calculated assuming an infinite depth of erosive material and a homogeneous particle-size distribution.

· ··

Pier/bent ¹ number	Station from ² left end of bridge (feet)	Contraction scour depth (feet)	Pier/bent scour depth without debris (feet)	Total ³ scour depth without debris (feet)
	500-year discharg	e is 21,000 cub	ic feet per second	
2	30	0.0	2.2	2.2
3	60	0.0^{4}	3.4	3.4
4	90	0.0 ⁴	6.3	6.3
5	120	0.0 ⁴	6.3	6.3
6	150	0.0^{4}	3.4	3.4
7	180	7.0	2.9	9.9
8	210	7.0	2.9	9.9
9	240	7.0	2.9	9.9
10	270	7.0	2.9	9.9
11	300	7.0	. 2.9	9.9
12	330	7.0	2.8	9.8
13	360	7.0	2.5	9.5

Table 4 Cumulative scour depths at piers/bents for the 500-year discharge at structure
124007200200 on Route SC 72, crossing the Sandy River in Chester County, South Carolina

¹ Pier/bent number corresponds to the South Carolina Department of Transportation (SCDOT) bridge plans.

² Stations are determined from left to right looking downstream.

³ Total scour depth is the sum of the contraction and pier/bent scour depths.

⁴ The calculated contraction scour is a negative value, but was set equal to zero to reflect a more reasonable estimate of scour during peak flood conditions.

NOTE: The SCDOT bridge plan borings (docket number 12.248) show subsurface rock and gravel deposits that could reduce the scour depths shown in the above table. For more information, see the SCDOT plans in report pocket.

NOTE: The pier and contraction scour equations used in this scour analysis were those recommended in Hydraulic Engineering Circular 18 (Richardson and others, 1993). Scour depths were calculated assuming an infinite depth of erosive material and a homogeneous particle-size distribution.

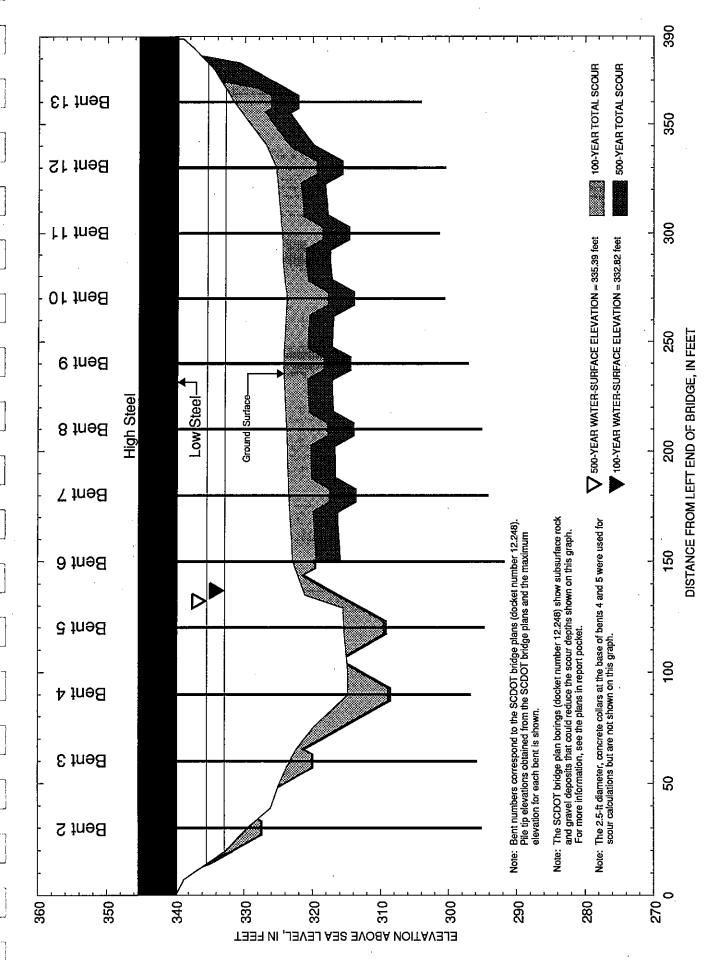


Figure 2.--Total scour depths for the 100- and 500-year discharges at the downstream face of structure 124007200200 on Route SC 72, crossing the Sandy River in Chester County, South Carolina.

• .

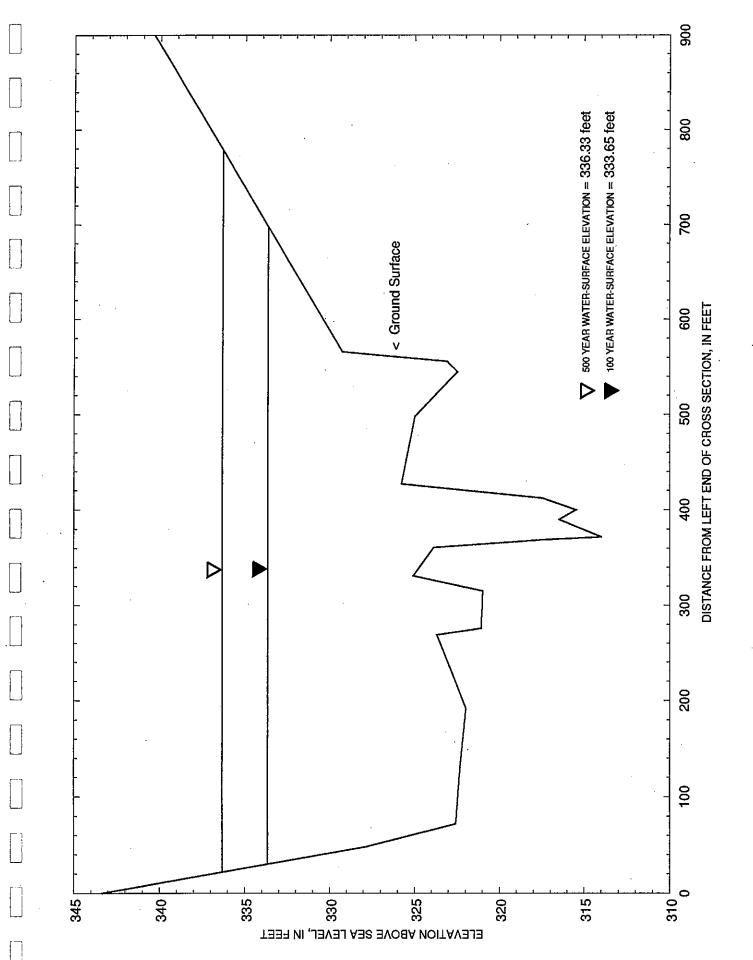


Figure 3.--Approach cross section at structure 124007200200 on Route SC 72, crossing the Sandy River in Chester County, South Carolina.

·

· · ·

.

.



Figure 4.--Structure 124007200200 on Route SC 72, crossing the Sandy River in Chester County, South Carolina as viewed from 145 ft upstream (April 20, 1993).

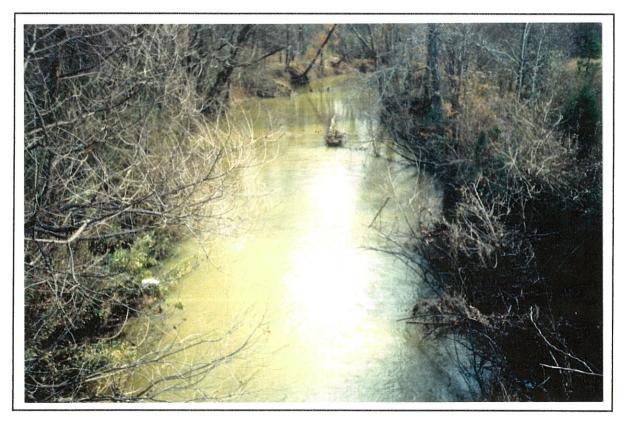


Figure 5.--Upstream channel as viewed from structure 124007200200 on Route SC 72, crossing the Sandy River in Chester County, South Carolina (January 28, 1991).

.

.

[]



Figure 6.--Downstream channel as viewed from structure 124007200200 on Route SC 72, crossing the Sandy River in Chester County, South Carolina (January 28, 1991).

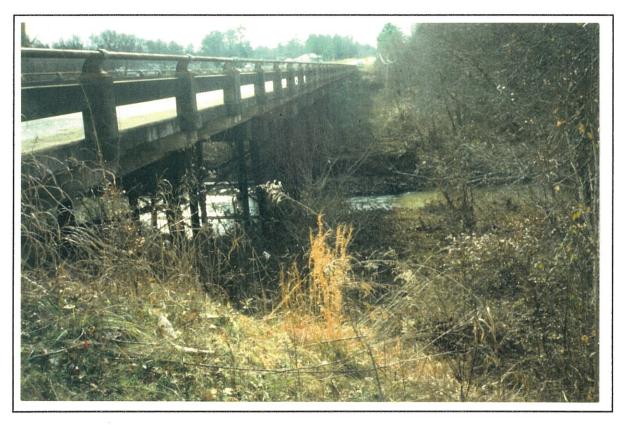


Figure 7.--Structure 124007200200 on Route SC 72, crossing the Sandy River in Chester County, South Carolina as viewed from the upstream, left abutment (January 28, 1991).

 $\left[\right]$

. .

SELECTED REFERENCES

- Arcement, G.J., Jr., and Schneider, V.R., 1989, Guide for selecting Manning's roughness coefficients for natural channels and flood plains: U.S. Geological Survey Water-Supply Paper 2339, 38 p.
- Arneson, L. A., Shearman, J. O., Jones, J. S., 1992, Evaluating scour at bridges using WSPRO: Transportation Research Board Draft Paper, 40 p.
- Bohman, L. R., 1990, Determination of flood hydrographs for streams in South Carolina: Volume 1. Simulation of flood hydrographs for rural watersheds in South Carolina: U.S. Geological Survey Water-Resources Investigations Report 89-4087, 53 p.
- Bohman, L. R., 1992, Determination of flood hydrographs for streams in South Carolina: Volume 2. Estimation of peak-discharge frequency, runoff volumes, and flood hydrographs for urban watersheds: U.S. Geological Survey Water-Resources Investigations Report 92-4040, 79 p.
- Froehlich, D. C., 1989, Local scour at bridge abutments in Ports, M. A., ed., Hydraulic Engineering--Proceedings of the 1989 National Conference on Hydraulic Engineering: New York, American Society of Civil Engineers, p. 13-18.
- Guimaraes, W. B., and Bohman, L. R., 1991, Techniques for estimating magnitude and frequency of floods in South Carolina, 1988: U.S. Geological Survey Water-Resources Investigation Report, 91-4157, 174 p.
- Gunter, H.E., Mason, R.R., and Stamey, T.C., 1987, Magnitude and frequency of floods in rural and urban basins in North Carolina: U.S. Geological Survey Water-Resources Investigations Report, 87-4096, 54 p.
- Laursen, E. M., 1960, Scour at bridge crossings: Journal of the Hydraulics Division, American Society of Civil Engineers, v. 86, no. HY2, p. 39-53.
- Laursen, E. M., 1963, An analysis of relief bridge scour: Journal of the Hydraulics Division, American Society of Civil Engineers, v. 89, no. HY3, p. 93-118.
- Richardson, E. V., Harrison, L. J., Richardson, J. R., and Davis, S. R., 1993, Evaluating scour at bridges: Federal Highway Administration Hydraulic Engineering Circular No. 18, Publication FHWA-IP-90-017, 131 p.
- Richardson, E. V., Simons, D. B., and Julien, P. Y., 1990, Highways in the river environment: Federal Highway Administration Publication FHWA-HI-90-016.
- Richardson, E. V., Simons, D. B., Karaki, S., Mahmood, K., and Stevens, M. A., 1975, Highways in the river environment: hydraulic and environmental design considerations: Federal Highway Administration.
- Shearman, J. O., 1990, User's manual for WSPRO-a computer model for water surface profile computations: Federal Highway Administration Publication FHWA-IP-89-027, 187 p.
- Shearman, J. O., Kirby, W. H., Schneider, V. R., and Flippo, H. N., 1986, Bridge waterways analysis model; research report: Federal Highway Administration Publication FHWA-RD-86-108, 112 p.
- U.S. Geological Survey, Interagency Advisory Committee on Water Data, 1982, Guidelines for determining flood flow frequency, Bulletin 17B of the Hydrology Subcommittee, 190 p.

SCOUR REPORT SUN	/IMAKY
------------------	--------

Structure Nui	mbe r	124007200200	Stream	Sandy Riv	/er	
County	Chester		Road	SC 72	District	4

Description of Bridge

Bridge length <u>390</u> ft Bridge width <u>33</u> ft Max span length <u>30</u> ft
Alignment of bridge to road (on curve or straight)straight
Abutment typespillthrough Embankment typesloping
Riprap on abutment? yes Date of inspection 04-08-1993
Description of riprap Both abutments are protected with 6- to 18-inch rock and asphalt chunks.
Brief description of piers/pile bents The bridge is supported by 12 interior bents, each
consisting of four 0.80-ft by 0.85-ft steel H-piles. Additionally, bents 4 and 5 have 2.5-ft-
diameter concrete collars at the base of their piles.
Is bridge skewed to flood plain according to USGS topo map? <u>no</u> Angle <u>0</u>
Is bridge located on a bend in channel? <u>no</u> If so, describe (mild, moderate, severe)

Debris accumulation on bridge at time of Level I or Level II site visit:

	Percent of channel blocked horizontally	Percent of channel blocked vertically
1-28-1991	45	2
)4-08-1993	0	0
······································		t brush and debris
	······································	

Describe any features near or at the bridge that may affect flow (include observation date). None observed.

.

Description of Flood Plain

l

General top	ographyIn the vicinity of the stu	ady area, the left flood plain is	
approximat	tely 400 ft wide and the right flood	plain rises quickly.	
Flood-plai	n conditions at bridge site: downst	tream (D/S), upstream (U/S)	
Date of ins	pection 04-08-1993		
D/S left:	Moderate hardwood cover with	some underbrush	
D/S r ight:	Moderate hardwood cover with s	some underbrush	
U/S left:	Moderate hardwood cover with s	ome underbrush	
U/S right:	Moderate hardwood cover with	some underbrush	
	Description of	f Channel	
Average top	p width <u>68</u> ft	Average depth9.4	ft
Predomina	nt bed material sand	Bank material silty sand	
Stream typ	e (straight, meandering, braided, st	wampy, channelized)meandering	
	· · · · · · · · · · · · · · · · · · ·		
Vegetative	cover on channel banks near bridge	e: Date of inspection <u>04-08-1993</u>	
D/S left:	Moderate hardwood cover with	trees leaning over channel	
D/S r ight:	Moderate hardwood cover with	trees leaning over channel	
U/S left:	Moderate hardwood cover with	trees leaning over channel	
U/S ri ght:	Moderate hardwood cover with t	rees leaning over channel	
Do b <mark>an</mark> ks a	ppear stable? <u>no</u> If not, d	lescribe location and type of instabili	ty a
date of obs	ervation. There are many points	of bank failure and moderate to heavy	7
fluvial ero	sion causing roots to be exposed a	nd trees to lean over the channel.	• • · · · · ·
		· · · · · · · · · · · · · · · · · · ·	
Desc r ibe an	y obstructions in channel and date	e of observation. None observed.	

.

٠.

<u>Hydrology</u>

Drainage area <u>105</u> mi²

Percentage of drainage area in physiographic provinces:

Physiographic province	Percent of drainage area						
Piedmont (high flow)	100						
Is drainage area considered rural or							
urbanization and potential for deve	lopment. The site encompasses a predominately						
rural drainage basin with little dev							
Is there a USGS gage on the stream of	of interest? <u>110</u>						
USGS gage des	cription						
USGS gage nut	nber						
Care Ansinger	area mi ²						
Is there a lake/pond that will signifi	cantly affect hydrology/hydraulics? <u>no</u>						
If so, describe							
Ca	culated Discharges						
$Q100 14,300 ft^3/s$	$Q500 \ 21,000 \ ft^3/s$						
Method used to determine discharges	The drainage basin is located within the "high-						
flow" area of South Carolina; theref	ore, the method described by C.L. Sanders (written						
commun., 11-1993) was used to com	pute flood discharges. In general, this method uses						
North Carolina USGS flood dischar	ge equations (WRIR 87-4096) to compute the 100-						
	500-year discharge using the 2-, 10-, and 100-year						
discharges (USGS Bulletin 17B, p.5							

Brief Description of the Water-Surface Profile Model (WSPRO) Analysis

Datum for WSPRO analysis (USGS survey, sea level, SCI	OOT plans	;)sea level
Datum tie between USGS survey and SCDOT plans	Add 0.2	ft to USGS survey to
obtain SCDOT plans' datum (docket number 12.248).		
Description of reference marks used to determine USGS	datum.	USGS Benchmark C
27 1933 434 (elevation=434.18 ft above sea level) is locate		kimately 3,200 ft
northeast of the Route SC 72 crossing of the Sandy River	r. RM1 is	a chiseled square on
the upstream, left headwall and has a surveyed elevation		
square on the downstream, right headwall and has a su	rveyed ele	evation of 342.65 ft.

[*] Cross section ID	Section Reference Distance (SRD) in feet	**How cross section was developed	Comments
SYNTA	-550	2 & 3	Starting cross section
SYNTB	-540	2&3	Transition from wooded area to pasture area
EXIT	-390	3	Exit cross section
FULV	0	3	Full valley cross section
BRIDG	0	1	D/S bridge face
APPRO	423	2 & 3	Approach cross section
		t	
	- <u>-</u>	,	<u> </u>
		Salardar Trac Solar Andrew Martin	
			<u>\$4.44,4.488,4.,</u>

Cross Sections Used in WSPRO Analysis

For location of cross sections see topographic map included with report (figure 1). For more detail on how cross sections were developed see WSPRO input file. *

** Cross section development: 1) survey at SRD 2) shift of survey data to SRD 3) modification of survey data based on topographic map 4) synthesized by combining channel survey data and topographic contours 5) other

Description of data and assumptions used in developing WSPRO model.

The Sandy River has a relatively uniform flood plain width in the study area, with no downstream natural or man-made contractions of flow that cause significant backwater at the Route SC 72 crossing. Therefore, it was assumed that slope-conveyance methodology would be adequate for estimating the starting water-surface elevation for the water-surface profile computations.

For this study, the WSPRO model requires, as a minimum, an exit section one bridge width downstream of the bridge, a full-valley section at the downstream face of the bridge, the bridge section, and an approach section one bridge width upstream of the bridge. Cross sections at the upstream and downstream faces of the bridge were directly surveyed and the more constricted (downstream) bridge face was used in the WSPRO model. The section reference distance (SRD) at the downstream face of the bridge was set to zero. An exit cross section was surveyed 382 ft downstream of the downstream bridge face, and an approach cross section was surveyed 145 ft upstream of the upstream bridge face. These cross sections were shifted by the channel slope to the appropriate SRD to represent the exit, full-valley, and approach cross sections required by the WSPRO model. These cross sections were extended using the slope of the contour lines from the USGS topographic map. In addition, section SYNTB was added 540 ft downstream to simulate the transition from pasture to forest on the right flood plain.

Bridge Hydraulics

Average embankment elevation <u>342.3</u> ft Average low steel elevation <u>339.7</u> ft

100-year discharge14,300 ft^3/s Water-surface elevation at D/S bridge face332.82ftArea of flow at D/S bridge face3,297 ft^2 Average velocity in bridge opening4.34ft/sMaximum WSPRO tube velocity at bridge6.19ft/s

Water-surface elevation at Approach section with bridge $\underline{333.65}$ ft Water-surface elevation at Approach section without bridge $\underline{333.12}$ ft Amount of backwater caused by bridge $\underline{0.53}$ ft

500-year discharge21,000 ft^3/s Water-surface elevation at D/S bridge face335.39ftArea of flow at D/S bridge face4,212 ft^2 Average velocity in bridge opening4.99ft/sMaximum WSPRO tube velocity at bridge6.95ft/s

Water-surface elevation at Approach section with bridge 336.33 ft Water-surface elevation at Approach section without bridge 335.69 ft Amount of backwater caused by bridge 0.64 ft

<u>Scour</u>

Describe any special assumptions or considerations made in bridge scour analysis.

Scour depths were computed using engineering judgement and the general guidelines described in Hydraulic Engineering Circular 18 (Richardson and others, 1993) and the Transportation Research Board Draft Paper, "Evaluating scour at bridges using WSPRO" (Arneson and others, 1992). Scour depths were calculated assuming an infinite depth of erosive material and a homogeneous particle-size distribution. The results of the scour analysis are presented in tables 1 through 4 and a graph of the scour depths is shown on figure 2.

The local pier scour was determined using the Colorado State University pier scour equation (Richardson and others, 1993). Bent 2 is located on the left overbank and was analyzed using the maximum left overbank WSPRO tube velocity and the depth of flow at that bent. Bents 7 through 13 are located on the right overbank and were analyzed using the maximum right overbank WSPRO tube velocity and the depth of flow at each bent. Bents 3 and 6 are located near the top of the banks and were analyzed as if they were in the channel to account for the possibility of a shift in the channel during a flood. Bents 4 and 5 are located in the channel and have 2.5-ft-diameter concrete collars at the base of the piles. The 2.5-ft collar width was used as the pile width and a pier length of 10 ft, using the cumulative collar widths, was used for bents 4 and 5 as recommended in Hydraulic Engineering Circular 18 (Richardson and others, 1993). Bents 3, 4, 5, and 6 were analyzed using 90 percent of the maximum WSPRO tube velocity and the maximum depth within the channel at the bridge. The maximum depth within the channel was used to account for possible changes in the thalweg during a flood.

The right overbank at the bridge was analyzed for contraction scour using Laursen's clear-water contraction scour equation (Richardson and others, 1993). The channel contraction scour was analyzed using Laursen's modified live-bed contraction scour equation (Richardson and others, 1993). There is no left overbank at the bridge, so contraction scour was not computed.

The live-bed contraction scour equation indicates the deposition of sediment in the channel at the bridge during the 100- and 500-year floods. (See negative scour values determined in scour calculations included at the end of the report). However, it seems unreasonable to expect sediment deposition at the bridge during peak flood conditions. Therefore, the negative scour values were set equal to zero as reflected in tables 1 through 4 and figure 2.

No abutment scour computations were made because the abutments are protected by riprap.

It should be noted that the SCDOT bridge plan borings (file number 12.248) show subsurface rock and gravel deposits that could affect the scour depths shown in this study. For more information, see the SCDOT bridge plans in the pocket at the back of the report.

WSPRO INPUT FILE

 $\left[\right]$

ł

ļ

T1 T2 *	Sandy River at SC 72 in Chester County Structure 124007200200 JMS 2 September 1994
* Q SK *	Q100 Q500 14300 21000 0.0015 0.0015
* * * *	Section TEMP1 was surveyed 382 ft downstream of the downstream bridge face. The right end of the cross section was extended by drawing the cross section on the USGS topographic map and adding station 990 that corresponds to the 340 ft contour. The distance was determined from the USGS field survey notes.
* GR GR GR GR GR GR *	TEMP1 -382 0.0015 0 336.5 41 332.8 1C0 329.1 139 324.3 167 324.6 216 322.8 274 323.4 318 324.3 373 325.0 454 325.0 509 324.8 521 319.9 530 316.4 539 314.4 551 315.0 564 314.7 572 316.4 579 322.1 618 322.2 658 324.3 687 325.2 727 324.6 765 326.6 852 329.9 916 332.9 938 334.2 990 340.0 340.0 326.6 852 329.9 916 332.9
* * * XS	Section SYNTA is used to model the wooded area on the right flood plain 550 ft downstream of the bridge.
GT N SA *	0.17 0.045 0.17 509 579
* * *	Section SYNTB is used to model the change from wooded area to pasture 540 ft downstream of the bridge.
XS GT	SYNTB -540
N SA	0.17 0.045 0.05 509 579
xs GT *	EXIT -390
XS GT *	FULV 0
* *	Downstream Bridge Face
BR GR GR GR GR GR GR SA CD	BRIDG 0 339.7 0 339.7 1 339.7 1.1 339.7 7 338.7 20 332.6 30 329.5 39 326.1 49 325.0 60 323.3 65 322.4 70 321.2 75 319.9 81 317.8 90 314.8 100 314.8 110 315.1 120 315.4 129 315.5 135 321.1 150 322.9 180 323.5 210 323.8 240 324.3 270 323.8 285 324.4 300 324.5 330 325.4 340 326.8 360 331.6 375 334.5 385 337.6 388.9 339.0 389 339.7 390 339.7 0 339.7 0.045 0.045 0.045 57 150 3 33 2 342.3

WSPRO INPUT FILE --Continued

KD	•	* * * 280	670						
PW 1		314.8 2.5	315.4 2.5	5 31	5.4 5.	0 318	.3 5.0	318.	.3 3.35
PW 1		318.9 3.35			2.9 1.		.9 2.55		4 2.55
PW 1		323.4 4.25	323.8 4.2				.3 5.95	324.	.3 6.8
PW 1		324.5 6.8	324.5 7.6	55 32	5.4 7.	65 325	.4 8.5	329.	5 8.5
PW 1		329.5 9.35	331.6 9.3	35 33	1.6 10.	2 339	.7 10.2	339.	7 0.0
*									
*	S	Section TEMP	2 was surve	eyed 1	.45 ft u	pstrea	m of the	upstr	ream bridge
*	j	face. The d	istance was	s dete	rmined	from t	he USGS :	field	survey notes.
*	5	The right en	d of the cr	coss s	ection	was ex	tended by	y draw	ving the cross
*		section on t				and a	dding sta	ation	900 that
*	C	corresponds	to the 340	ft co	ntour.				
*									
	TEMP2								~~~ ~
GR.					322.2		321.9		321.6
GR		269 323.3					324.7		323.5
GR					316.1				317.1
GR		127 325.4	498 324.6	545	322.1	556	322.7	566	328.9
GR	ç	900 340.0							
*		400							
	APPRO	423							
GT		0.15 0.0	45 0.14						
N SA		361	45 0.14						
BP		301	427						
ЫГ *		J01							
	BRIDG	332 82 3	32.82,14300						
	BRIDG		32.86,14300						
	APPRO		33.65,14300						
	APPRO		33.65,14300				-		
	BRIDG		35.39,21000						
	BRIDG		35.43,21000						
	APPRO		36.33,21000						
HP 2 2			36.33,21000						
*		- •	-				•		

EX ER

WSPRO OUTPUT

 WSPRO
 FEDERAL HIGHWAY ADMINISTRATION - U. S. GEOLOGICAL SURVEY

 V060188
 MODEL FOR WATER-SURFACE PROFILE COMPUTATIONS

 Sandy River at SC 72 in Chester County
 Structure 124007200200

 JMS 2 September 1994
 *** RUN DATE & TIME: 09-07-94 13:42

 CRCSS-SECTION PROPERTIES:
 ISEQ = 5; SECID = BRIDG; SRD =
 0.

 WSEL SA#
 AREA
 K TOPW
 WETP ALPH
 LEW
 REW
 QCR

 1
 203.
 20293.
 37.
 39.
 2684.

 2
 1367.
 264830.
 93.
 97.
 29740.

 3
 1726.
 227583.
 216.
 217.
 27661.

 332.32
 3296.
 512706.
 347.
 352.
 1.14
 20.
 366.
 54087.

WSPRO FEDERAL HIGHWAY ADMINISTRATION - U. S. GEOLOGICAL SURVEY V060188 MODEL FOR WATER-SURFACE PROFILE COMPUTATIONS

> Sandy River at SC 72 in Chester County JMS 2 September 1994 *** RUN DATE & TIME: 09-07-94 13:42

Ο. VELOCITY DISTRIBUTION: ISEQ = 5; SECID = BRIDG; SRD =
 WSEL
 LEW
 REW
 AREA
 K
 Q
 VEL

 332.86
 19.4
 366.5
 3309.8
 515881.
 14300.
 4.32
 19.4 61.2 75.8 85.0 91.8 98.2 X STA. 244.2166.0136.3118.1116.82.934.315.256.056.12 A(I) V(I) 98.2 104.7 111.3 118.0 125.1 133.2 X STA. 115.5119.1117.2124.1132.06.196.006.105.765.42 A(I) V(I) X STA. 133.2 147.9 165.7 183.9 203.0 223.0 164.8 174.9 172.7 176.2 180.1 A(I) 3.97 4.09 4.14 4.06 4.34 V(I) 223.0 244.3 265.5 285.7 309.1 366.5 X STA. 184.6 186.6 178.5 194.8 307.5 A(I) 3.83 4.01 3.67 2.33 3.87 V(I)

WSPRO FEDERAL HIGHWAY ADMINISTRATION - U. S. GEOLOGICAL SURVEY V060188 MODEL FOR WATER-SURFACE PROFILE COMPUTATIONS

4. 5

Sandy River at SC 72 in Chester County Structure 124007200200 JMS 2 September 1994 *** RUN DATE & TIME: 09-07-94 13:42 CROSS-SECTION PROPERTIES: ISEQ = 6; SECID = APPRO; SRD = 423.

WSEL	SA#	AREA	к	TOPW	WETP	ALPH	LEW	REW	QCR
	1	3469.	164247.	331.	333.				63744.
	2	1053.	207752.	66.	72.				23865.
	3	1538.	51873.	271.	273.				20804.
333.65		6061.	423872.	668.	679.	4.11	30.	698.	51125.

WSPRO FEDERAL HIGHWAY ADMINISTRATION - U. S. GEOLOGICAL SURVEY V060188 MODEL FOR WATER-SURFACE PROFILE COMPUTATIONS

> Sandy River at SC 72 in Chester County Structure 124007200200 JMS 2 September 1994 *** RUN DATE & TIME: 09-07-94 13:42

VE	LOCITY DIST	RIBUTI	ON: ISEQ =	6; SECID	= APPRO;	SRD = 4	23.
				REA 1 0.6 423872			
		573.5	415.1	430.5	420.2	211.3 435.0 1.64	
 STA. A(I) V(I)	,	419.3	413.7		126.6	372.8 88.9 8.04	
STA. A(I) V(I)		94.2	96.4	98.5	95.9	399.0 93.5 7.65	
 STA. A(I) V(I)		99.6	113.5	316.8	479.1	510.1 841.2 0.85	

FEDERAL HIGHWAY ADMINISTRATION - U. S. GEOLOGICAL SURVEY WSPRO MODEL FOR WATER-SURFACE PROFILE COMPUTATIONS V060188 Sandy River at SC 72 in Chester County Structure 124007200200 JMS 2 September 1994 *** RUN DATE & TIME: 09-07-94 13:42 CROSS-SECTION PROPERTIES: ISEQ = 5; SECID = BRIDG; SRD = Ο. WETP ALPH WSEL SA# AREA K TOPW LEW REW QCR 306. 36561. 43. 45. 4646. 1 97. 2 1606. 346417. 93. 37872. 41411. 3 2298. 354015. 228. 229. 378. 77186. 335.39 4211. 736993. 364. 370. 1.11 14.

FEDERAL HIGHWAY ADMINISTRATION - U. S. GEOLOGICAL SURVEY WSPRO MODEL FOR WATER-SURFACE PROFILE COMPUTATIONS V060188

> Sandy River at SC 72 in Chester County Structure 124007200200 JMS 2 September 1994 *** RUN DATE & TIME: 09-07-94 13:42

VE:	LOCITY DISTRIBUTI	ON: ISEQ =	5; SECID	= BRIDG;	SRD =	0.
	WSEL LEW 335.43 14.0					
 STA. A(I) V(I)	14.0 309.1 3.40	214.8	175.6	158.0	151.0	
STA. A(I) V(I)	99.1 154.3 6.80	153.6	155.9	163.3	202.5	
STA. A(I) V(I)	143.8 208.3 5.04	213.7	212.0	219.6	220.7	
STA. A(I) V(I)	234.0 221.8 4.73	220.5	220.8	229.7	419.7	

WSPRO FEDERAL HIGHWAY ADMINISTRATION - U. S. GEOLOGICAL SURVEY V060188 MODEL FOR WATER-SURFACE PROFILE COMPUTATIONS

.

Sandy River at SC 72 in Chester County Structure 124007200200 JMS 2 September 1994 *** RUN DATE & TIME: 09-07-94 13:42

CROSS-SECTION PROPERTIES: ISEQ = 6; SECID = APPRO; SRD = 423.

WSEL	SA#	AREA	ĸ	TOPW	WETP	ALPH	LEW	REW	QCR
	1	4367.	236937.	339.	342.				88922.
	2	1230.	269118.	66.	72.				30125.
	3	2372.	89836.	352.	353.				34973.
336.33		7969.	595891.	757.	768.	4.12	22.	779.	72337.

WSPRO FEDERAL HIGHWAY ADMINISTRATION - U. S. GEOLOGICAL SURVEY V060188 MODEL FOR WATER-SURFACE PROFILE COMPUTATIONS

> Sandy River at SC 72 in Chester County Structure 124007200200 JMS 2 September 1994 *** RUN DATE & TIME: 09-07-94 13:42

VELOCITY DISTRIBUTION: ISEQ = 6; SECID = APPRO; SRD = 423.

WSEL LEW REW AREA K Q VEL 336.33 21.8 778.5 7969.4 595891. 21000. 2.64

STA. A(I) V(I)		517.7	508.2	171.2 505.3 2.08	526.5	245.3
		488.8	561.7	362.1 175.1 6.00	115.1	377.3
 A(I)	120.5	123.4	123.4	395.2 122.9 8.54	122.4	
 A(I)	130.7	197.9	556.2	479.7 547.9 1.92	1252.3	778.5

. . .

WSPRO V060188			HWAY ADMI FOR WATE					AL SURVEY TIONS	
San JMS	2 Sep	tember					ructure	124007200	200
	*** RUN	DATE &	TIME: 09		4 T2:4	: 2			
	SRDL FLEN	LEW REW	AREA K		HF HO	EGL ERR			WSEL
SYNTA:XS -550.	* * * * * * * * * * * *	49. 903.	6267. 369140.	0.51 6.31	* * * * * * * * * *	332.55 ******	327.53 0.37	14300. 2.28	332.04
===135 CON	VEYANCE	RATIO C				D LIMITS = 1.4			
SYNTB:XS -540.	10. 10.	45. 909.	6506. 529163.	0.23 3.01	0.01 0.00	332.56 0.00	****** 0.24	14300. 2.20	332.34
EXIT :XS -390.	150. 150.	46. 907.	6408. 518803.	0.23 3.02	0.11 0.00	332.68 0.00	****** 0.25	14300. 2.23	332.45
FULV :FV 0. <<	390.	901.	493231.	3.04	0.01	0.00	0.26	14300. 2.32 ED) FLOW>	
APPRO:AS 423. <<	423.	682.	393730.	4.10	0.07	0.00	0.30		
	<<< <re< td=""><td>SULTS F</td><td>REFLECTIN</td><td>G THE</td><td>CONSTR</td><td>ICTED FI</td><td>OW FOLL</td><td><<<<w< td=""><td></td></w<></td></re<>	SULTS F	REFLECTIN	G THE	CONSTR	ICTED FI	OW FOLL	<<< <w< td=""><td></td></w<>	
XSID:CODE SRD	SRDL FLEN	LEW REW	AREA K	VHD ALPH	HF HO	EGL ERR		Q VEL	WSEL
BRIDG:BR 0.			3297. 512984.			333.26 0.01		14300. 4.34	332.82
TYPE PI 3.	PCD FLOW 1. 1.	C 0.822	P/A 0.032	LSE 339.7	EL BL 70 ****	EN XLA ** ****	B XRAI		
XSID:CODE SRD	SRDL FLEN		AREA K					Q VEL	WSEL
APPRO:AS 423.			6062. 424022.						333.65
M(G) 0.467	M(K) 0.278	к 305390	Q XLKQ . 280.	XRK 670	(Q 0). 33	TEL 3.21			

<<<<END OF BRIDGE COMPUTATIONS>>>>>

FEDERAL HIGHWAY ADMINISTRATION - U. S. GEOLOGICAL SURVEY WSPRO MODEL FOR WATER-SURFACE PROFILE COMPUTATIONS V060188 Sandy River at SC 72 in Chester County Structure 124007200200 2 September 1994 JMS *** RUN DATE & TIME: 09-07-94 13:42 AREA HFEGL CRWS WSEL XSID:CODE SRDL LEW VHD 0 VEL SRD FLEN REW K ALPH HO ERR FR# ***** 8605. 0.57 ***** 335.22 329.05 21000. SYNTA:XS 334.66 18. 944. 541824. 6.12 ***** ****** -550. ***** 0.35 2.44 ===135 CONVEYANCE RATIO OUTSIDE OF RECOMMENDED LIMITS. "SYNTB" KRATIO = 1.518905. 0.24 0.01 335.23 ******* 21000. 334.99 SYNTB:XS 10. 14. 947. 816852. 2.74 0.00 0.00 -540. 10. 0.22 2.36 150. 15. 8790. 0.24 0.10 335.34 ****** 21000. 335.10 EXIT :XS -390. 150. 946. 801688. 2.75 0.00 0.00 2.39 0.23 8498. 0.26 0.28 335.63 ****** 21000. FULV :FV 390. 19. 335.37 Ο. 943. 763854. 2.78 0.01 0.00 0.24 390. 2.47 <<<<THE ABOVE RESULTS REFLECT "NORMAL" (UNCONSTRICTED) FLOW>>>>> 7491, 0,50 0.44 336.19 ****** 21000. 335.69 423. 24. APPRO:AS 423. 0.00 0.31 759, 551785, 4,11 0,12 423. 2.80 <<<<THE ABOVE RESULTS REFLECT "NORMAL" (UNCONSTRICTED) FLOW>>>>> <<<<RESULTS REFLECTING THE CONSTRICTED FLOW FOLLOW>>>>> XSID:CODE SRDL LEWAREA VHD HFEGL CRWS WSEL Q REW K ALPH HO ERR FR# SRD FLEN VEL 14. 4212. 0.58 0.50 335.97 328.46 21000. 335.39 BRIDG:BR 390. Ο. 378, 737300, 1.50 0.14 390. 0.00 0.32 4.99 XRAB TYPE PPCD FLOW С P/A LSEL BLEN XLAB 0.816 0.031 339.70 ****** ****** ***** 3 1. 1. XSID:CODE SRDL LEW AREA VHD HFEGL CRWS WSEL 0 SRD FLEN REW K ALPH HO ERR FR# VEI. APPRO:AS 390. 22. 7968. 0.44 0.76 336.77 328.71 21000. 336.33 444. 778. 595766. 4.12 0.04 0.00 0.29 423. 2.64 M(G) M (K) KQ XLKQ XRKQ OTEL 0.505 0.277 430757. 280. 670. 335.84

ER

NORMAL END OF WSPRO EXECUTION.

<<<<END OF BRIDGE COMPUTATIONS>>>>

PIER SCOUR COMPUTATIONS FOR Sandy River at SC 72 in Chester County, SC Structure 124007200200 Q100 scour computations including bents 2 through 9 JMS 8 Sept. 1994 HYDRAULIC VARIABLES USED IN CSU EQUATION

PIER NUMBER	2	3	4	5	6	7	8	9
PIER STATION (FT)	30	60	90	120	150	180	210	240
LOCATION OF PIER	lfp	ltb	mcl	mcr	rtb	rfp	rfp	rfp
Y1: DEPTH (FT)	3.4	18.1	18.1	18.1	18.1	9.4	9.1	8.6
V1: VEL. (FPS)	2.9	5.6	5.6	5.6	5.6	4.1	4.1	4.1
a: PIER WIDTH (FT)	0.9	0.9	2.5	2.5	0.9	0.9	0.9	0.9
L: PIER LENGTH (FT)	3.4	3.4	10.0	10.0	3.4	3.4	3.4	3.4
PIER SHAPE	1	1	2	2	1	1	1	1
ATTACK ANGLE	0	0	0	0	0	0	0	0
K1 (SHAPE COEF.)	1.10	1.10	1.00	1.00	1.10	1.10	1.10	1.10
K2 (ANGLE COEF.)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FROUDE NO.	0.28	0.23	0.23	0.23	0.23	0.24	0.24	0.25
	COMPUTED	SCOUR DE	PTHS USI	NG CSU EQU	JATION			
SCOUR DEPTH (FT)	1.76	2.90	5.32	5.32	2.90	2.34	2.33	2.31
MAX SCOUR DEPTH (FT)	1.93	3.19	5.85	5.85	3.19	2.57	2.56	2.54

Q100 scour computations including bents 10 through 13 JMS 8 Sept. 1994

	Н	DRAULIC	VARIABLES	USED IN	CSU EQUATION
PIER NUMBER	10	11	12	13	-
PIER STATION (FT)	270	300	330	360	
LOCATION OF PIER	rfp	rfp	rfp	rfp	
Y1: DEPTH (FT)	9.1	8.4	7.5	1.3	
V1: VEL. (FPS)					
a: PIER WIDTH (FT)	0.9	0.9	0.9	0.9	
L: PIER LENGTH (FT)	3.4	3.4	3.4	3.4	
PIER SHAPE	1	1	1	1	
ATTACK ANGLE	0	0	0	0	
K1 (SHAPE COEF.)	1.10	1.10	1.10	1.10	
K2 (ANGLE COEF.)	1.00	1.00	1.00	1.00	
FROUDE NO.	0.24	0.25	0.27	0.64	
	COMPUTED	SCOUR I	EPTHS USIN	NG CSU EQ	QUATION
SCOUR DEPTH (FT)	2.33	2.30	2.27	1.79	
MAX SCOUR DEPTH (FT)	2.56	2.53	2.50	1.97	

"MAX SCOUR DEPTH" includes an additional 10 percent of the computed CSU scour depth as recommended in HEC 18

FOR

Sandy River at SC 72 in Chester County, SC Structure 124007200200 Q100 scour computations

JMS 8 Sept. 1994

LIVE-BED SCOUR COMPUTATIONS

DISCHARGE (CFS) BOTTOM WIDTH (FT) MANNINGS n AVERAGE DEPTH (FT)	MAIN CHANNEL 7010. 66.0 0.045 19.0	73:	
ENERGY SLOPE D50 (FT) FALL VELOCITY (FPS) K1 COEF. K2 COEF.		0.00170 0.0043 0.59 0.64 0.21	
COMPUTED DEPTH AT CONTRACT DEPTH AT MAIN CHANNEL (FT) DEPTH OF CONTRACTION SCOUR		= 16.7 = 19.0 = -2.3	

RIGHT OVERBANK IN BRIDGE OPENING CLEAR-WATER CONTRACTION SCCUR COMPUTATIONS

DISCHARGE IN CONTRACTED SECTION (CFS)	=	6350.
WIDTH OF CONTRACTED SECTION (FT)	-	190.0
MEDIAN GRAIN SIZE (FT)	-	0.0036
COMPUTED DEPTH OF CONTRACTED SECTION (FT)	-	13.0
AVERAGE FLOOD PLAIN DEPTH (FT)	-	9.6
DEPTH OF CONTRACTION SCOUR (FT)		3.3

PIER SCOUR COMPUTATIONS

FOR Charten Cau

Sandy River at SC 72 in Ch			
Q500 scour computations incl	-		
	HYDRAULIC VARIABLE	S USED IN	CSU EQUATION

			HID	RAOTIC AN	RIABLES 0	SED IN CSO	FOOVITON	
PIER NUMBER	2	3	4	5	6	7	8	9
PIER STATION (FT)	30	60	90	120	150	180	210	240
LOCATION OF PIER	lfp	ltb	mcl	mcr	rtb	rfp	rfp	rfp
Y1: DEPTH (FT)	5.9	20.6	20.6	20.6	20.6	11.9	11.6	11.1
V1: VEL. (FPS)	3.4	6.3	6.3	6.3	6.3	5.0	5.0	5.0
a: PIER WIDTH (FT)	0.9	0.9	2.5	2.5	0.9	0.9	0.9	0.9
L: PIER LENGTH (FT)	3.4	3.4	10.0	10.0	3.4	3.4	3.4	3.4
PIER SHAPE	1	1	2	2	1	1	1	1
ATTACK ANGLE	0	0	0	0	0	0	0	0
K1 (SHAPE COEF.)	1.10	1.10	1.00	1.00	1.10	1.10	1.10	1.10
K2 (ANGLE COEF.)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FROUDE NO.	0.25	0.24	0.24	0.24	0.24	0.26	0.26	0.27
,	COMPUTED	SCOUR DE	PTHS USI	NG CSU EQ	UATION			
SCOUR DEPTH (FT)	2.02	3.11	5.69	5.69	3.11	2.63	2.62	2.60
MAX SCOUR DEPTH (FT)	2.22	3.42	6.26	6.26	3.42	2.89	2.88	2.86

Q500 scour computations including bents 10 through 13 JMS 8 Sept. 1994 ▁▁▁▁▁ヹヹヹヹヺゔਗ਼ਖ਼ਖ਼^{ੑੑ}₩₽₽₽ਖ਼ਖ਼ਖ਼ੑਜ਼ੑਫ਼ਫ਼ਫ਼ੑਫ਼ਫ਼ਫ਼ਫ਼ਫ਼ਫ਼ਫ਼ਫ਼ਫ਼ਸ਼ਸ਼ਖ਼ਖ਼ਖ਼ਖ਼ਖ਼ਸ਼ਖ਼ਫ਼ਫ਼ਫ਼ਫ਼ਫ਼ਫ਼ੑਗ਼ਖ਼ਖ਼ਜ਼ੑੑਫ਼ਫ਼ਫ਼ਫ਼ਫ਼ਫ਼ਖ਼ਖ਼ਖ਼ਖ਼ਖ਼ੑਖ਼ਖ਼ਖ਼ _

	н	YDRAULIC	VARIABLES	USED IN	CSU EQUATION
PIER NUMBER	10	11	12	13	-
PIER STATION (FT)	270	300	330	360	
LOCATION OF PIER	rfp	rfp	rfp	rfp	
Y1: DEPTH (FT)	11.6	10.9	10.0	3.8	
V1: VEL. (FPS)			5.0		
a: PIER WIDTH (FT)	0.9	0.9	0.9	0.9	
L: PIER LENGTH (FT)	3.4	3.4	3.4	3.4	
PIER SHAPE	l	1	1	1	
ATTACK ANGLE	-	0	0	0	
K1 (SHAPE COEF.)	1.10	1.10	1.10	1.10	
K2 (ANGLE COEF.)	1.00	1.00	1.00	1.00	
FROUDE NO.	0.26	0.27	0.28	0.46	
	COMPUTEI	SCOUR I	DEPTHS USIN	IG CSU E	DUATION
SCOUR DEPTH (FT)	2.62		2.57	2.25	-
	2.88		2.82	2.48	

T

"MAX SCOUR DEPTH" includes an additional 10 percent of the computed CSU scour depth as recommended in HEC 18

32

-:

FOR

Sandy River at SC 72 in Chester County, SC Structure 124007200200 Q500 scour computations

JMS 8 Sept. 1994 .**__**_____

LIVE-BED SCOUR COMPUTATIONS

	MAIN CHANNEL	CONTRACTED	SECTION
DISCHARGE (CFS)	9480.	987	0.
BOTTOM WIDTH (FT)	66.0	8	6.3
MANNINGS n	0.045		0.045
AVERAGE DEPTH (FT)	21.6		
ENERGY SLOPE		0.00180	
D50 (FT)		0.0043	
FALL VELOCITY (FPS)		0.59	
K1 COEF.		0.64	
K2 COEF.		0.21	
COMPUTED DEPTH AT CONTRAC	CTED SECTION (FT)) = 18.8	
DEPTH AT MAIN CHANNEL (F	F)	= 21.6	
DEPTH OF CONTRACTION SCOU	UR (FT)	= -2.8	

RIGHT OVERBANK IN BRIDGE OPENING CLEAR-WATER CONTRACTION SCOUR COMPUTATIONS

DISCHARGE IN CONTRACTED SECTION (CFS)	=	10090.
WIDTH OF CONTRACTED SECTION (FT)	=	190.0
MEDIAN GRAIN SIZE (FT)	=	0.0036
COMPUTED DEPTH OF CONTRACTED SECTION (FT)	=	19.3
AVERAGE FLOOD PLAIN DEPTH (FT)	=	12.3
DEPTH OF CONTRACTION SCOUR (FT)	=	7.0

. . .

.

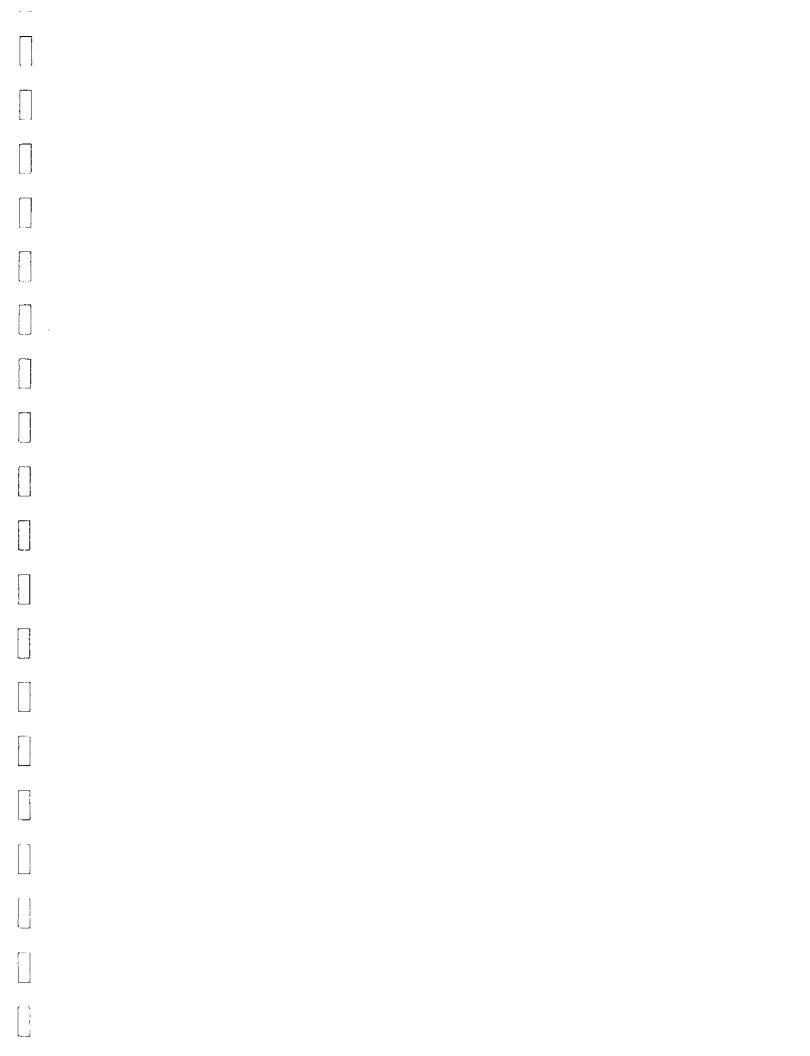
. . .

.

.

.

.



ļ

 $\left[\right]$



United States Department of the Interior



GEOLOGICAL SURVEY

Water Resources Division Stephenson Center, Suite 129 720 Gracern Road Columbia, SC 29210-7651

September 22, 1994

William H. Hulbert, P.E. Hydraulic Engineer South Carolina Department of Transportation 955 Park Street Columbia, South Carolina 29202

Dear Mr. Hulbert:

We are pleased to transmit to you another report of the Level II Bridge Scour Program titled, "Level II bridge scour analysis for structure 124007200200 on Route SC 72, crossing the Sandy River in Chester County, South Carolina," by J. Mike Sullivan and Andy W. Caldwell. The technical aspects of the report have been reviewed by the South Carolina District Surface-Water Specialist and the report has been approved by the South Carolina District Report Specialist.

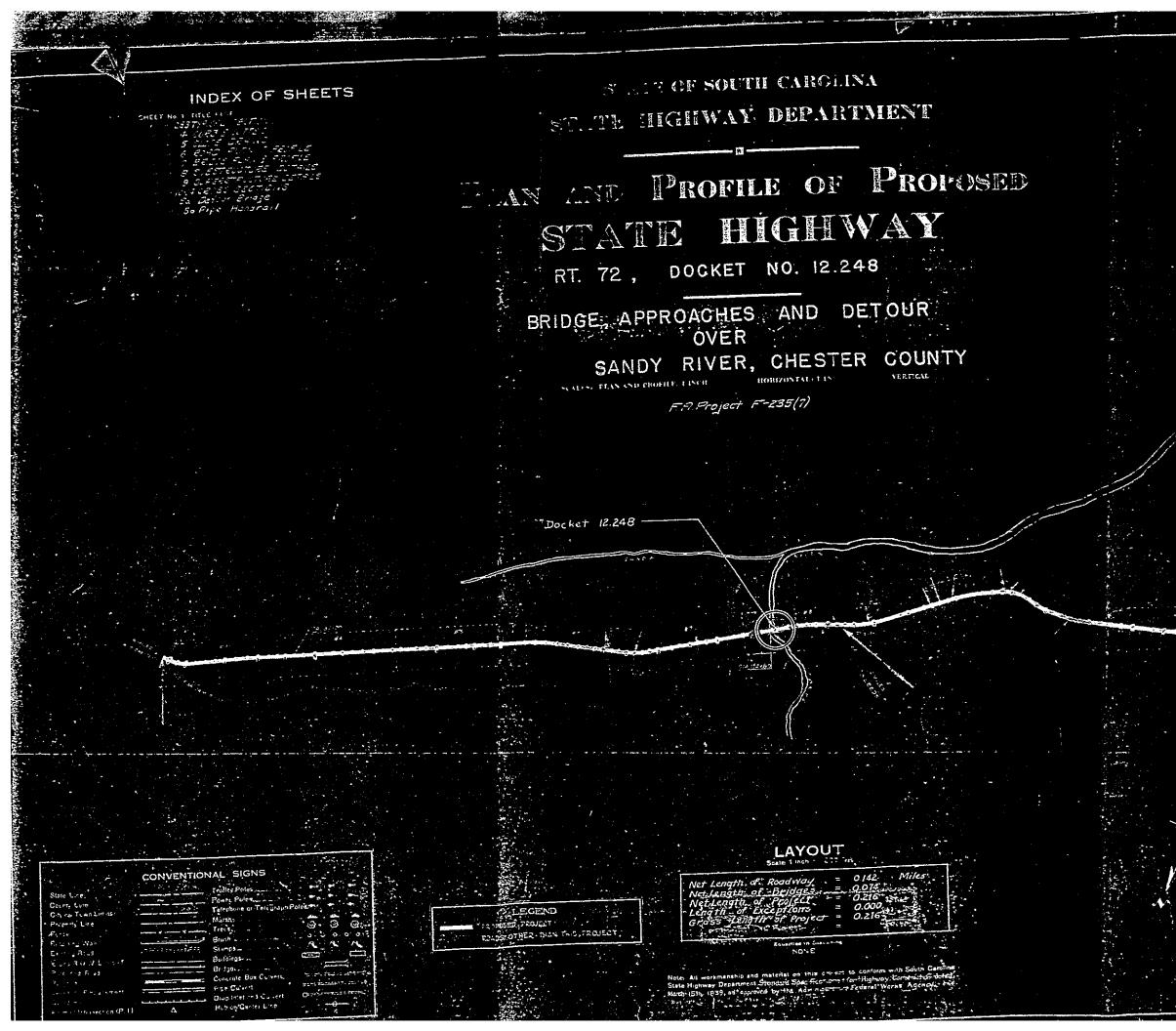
If you have any questions concerning this report please contact me (750-6165) or Andy Caldwell (750-6101) and we will be glad to assist you in any way possible.

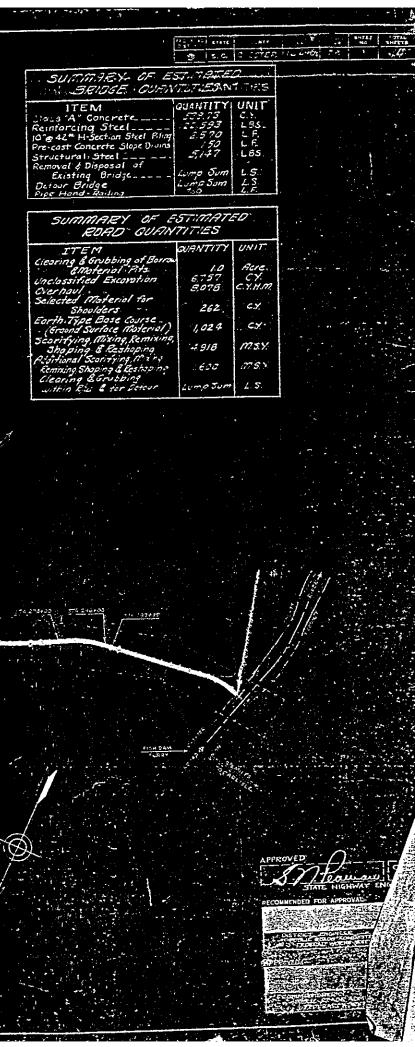
Sincerely,

J. Mike Sullivan, E.I.T. Civil Engineer

Enclosure





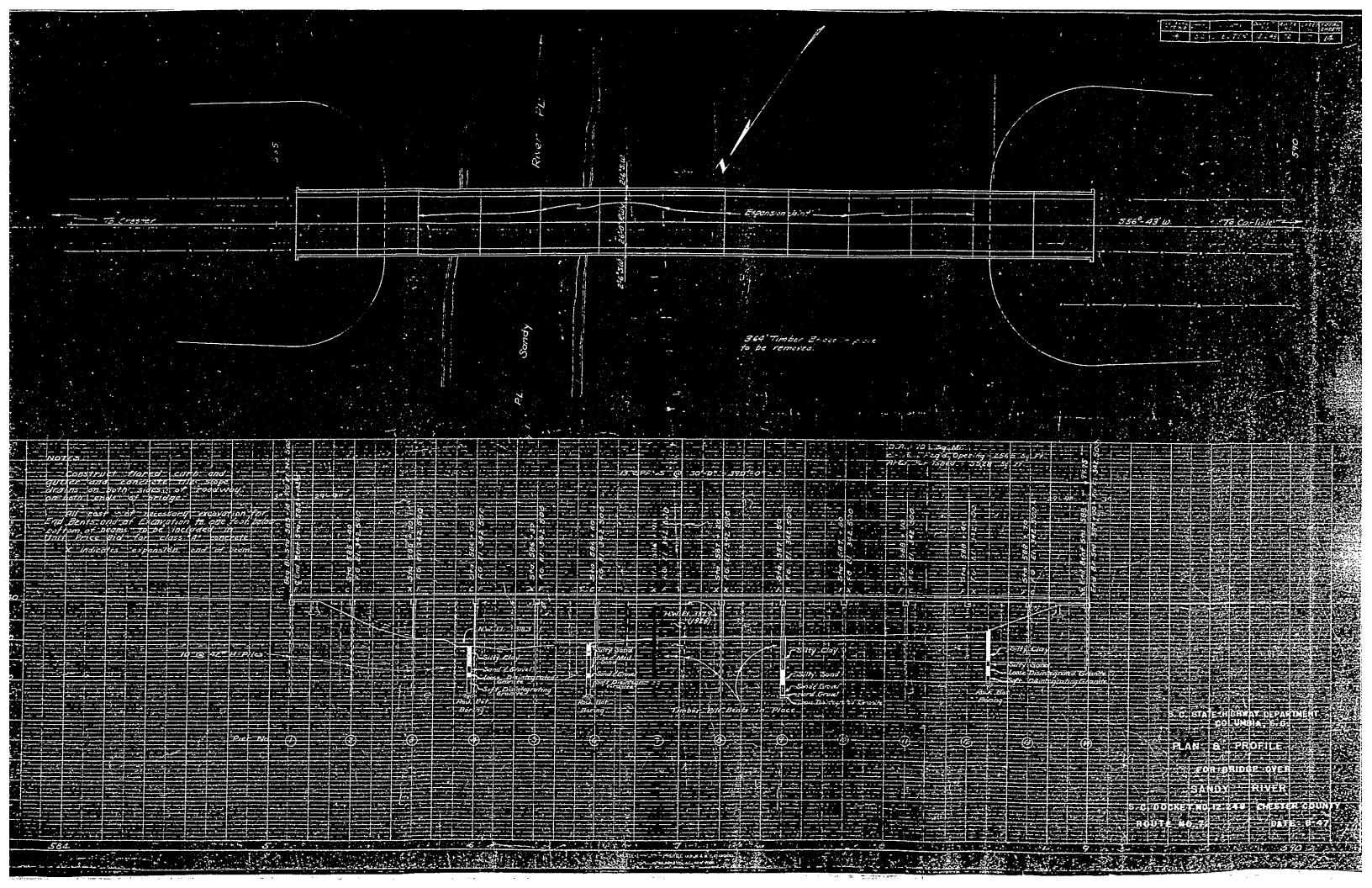


. .

· · · ·

а 1 .

: i . - ; ; .



. ×

.





PILE RECORD

5021	~~ *	-								>							2 990 #		FAL	151	EE	7	<u> </u>	PE 21	20 P	-		2
									÷			. WE	IGH <u>T</u> .				PAY	C.O. AT 0.	4 C.O. AT 0.	25 BEN	FOOT	ING PILE	DATE	MANETER		PENETRA-	- PEN, PEI BLOW	R BEA
A.+. 1+		ILE	DATE	DIAMET			PENETR	A PEN.	PER BEA	RING	ELEVATION PILE TIP	ELEVATION	IN PLAC	LENG	TH CUT	r - OFF	ENGTH	OF UNIT PH	4) C.O. AT 0.		·	NO.	DRIVEN	BUTT	v <u>≤</u> : • • • • • • • • • • • • • • • • • •	·		
NT F00			RIVEN	BUTI	<u>T T</u>		TION							1-1-1 AV	" M					_				11 A.				<u> </u>
		_+-		<u> </u>			ELI	4 PT	7 AJ	70"	0+42*	<u> </u>		a					~				<u> </u>		and the second			
				<u> </u>	-1-3						अनेवास्ट्रास्त्र भूरधान्त्राः च्याः					7.14	33.84 -											1-
	·	-	5-7-4				25.09			3.771	305.31 304.34	338.17	<u>37.36</u> 33.13	1 9 45.0	00/ 1	1.17 .	34.93 -		-T				<u> </u>		an an the second			二
+		2	5-6-41	<u></u>			26.06	1 1				"\\ 338.17 Vota/#4	32.54			0.04	31.06		, 						<u> </u>			1
			- 7 -4 - 7 -4				2.2.29		75	1.17 Y	301.0	Total BH 338-17	4 129.29	1 17au			<u>/33.29</u> <u>37.76</u>				<u> </u>							<u></u>
		2	5-8-41				2430			~ 10 V	2011/2			v #4		119	3451 -											+
		$\overline{-}$		+			21.12					338.17 Total 8t.		100	00 × 10		35.74-						· · · · · · · · · · · · · · · · · · ·		2.17 570×15	44.85].
1-		#	- 8-41	-			22.28		<u> </u>	A 17 .	300.62	\$38.17	37.48		00	7.52 1	38.48						ΡΞ	2 WH. 5+10	(~~) × mae			干
			5-10-4	-			24.42		00 V	54 <u>.50</u> :	291.00		10.92	~ ~	Y	1.18 V	44.82 3933	í		_							<u> </u>	_
		3					27.74		50 V 50 1	<u>5.17</u>	297.35	\$38.17	39.33	45.0	10 1 2	73.20 V	160.10	4			_	·			<u> </u>			<u></u>
		#	5-10-4	ar				_		3.150	18 301.64	338.17		× 45.	00 -	11.81 4	<u>3756</u> 39.19	<u></u>				-						
		1	5-10-4	<u></u>			22.7/	2 2	00 .	37.38	297.71		4.7	1 47	001	5.77 ×	41.23	<u> </u>				_	+	ļ	<u> </u>	I		+-
		3	5-10-4				26.00		<u>75 √</u> 25_√	35.1	29825	- 17-6 184		01 192.	.00 - 3	3610 V	159.90	· \				_	<u> </u>				1	+-
							259	z	100 1	3450	298.62	\$38.17	<u>39.5:</u> 39.4:	5. 30.		10.55 .	40.45	¥			-		<u> </u>			<u></u>		
2		2	<u>5-11-1</u>	<u>+</u>			23.7	<u>_</u>		<u>3578</u> 3571	1 300.14	7 "	37.33	50	00 V .	12.67 1	41.92			_				<u></u>			- <u> </u>	-
		3		48	+-		24.9		275 V	35./1	<u>~ ~~~~~</u>	Total 84	10 131.6	5 200	200 V	42.75 v 8.15 V	42.8	ř.							<u></u>			
			5-11-				28.7		275	3518	V 296.37 V 296.89	338.17	#1.33	v	╩╧┷╋╌	8.67	42.5	<u></u>	- • • 5						4,99 t			+-
7		_2		<u> </u>			24-3		200-V-L-	-1.1.50-	√_		41.92	1 60	00	0 10	1 41.94	· · I	<u> </u>	_								<u>+</u> _
7		3	5-11-	7 8			26.3	2			1 297.2	Votel Br	9 1400	200	0.00	6.19	44.8	<u> </u>	F			<u>``</u>						
			5-12-	#			29.2			26/1	2943	/ "	434	6		694	-43.8	4						<u> </u>		F		
8		2			<u> </u>		27.0				× 2753 × 274.0	339.1	44.	6 - 50	200.0	5.84	√ <u>+5.1</u> √ <u>171.</u>		, ,					ļ				<u> </u>
8		¥.	5-12-	4ľ	+		27.4		275 /				7 #6-	15 5	0.00	3.15	<u>× 47-1</u>	<u> </u>							·			
7			5-13-				31.3					7 11	46.	27 .	<u>H U</u>	4.73	V 462	7.51										<u> </u>
;		2	5-12-				27-3	<u>در</u>	275 1	35/1	1 2013	0 " 5) 338.17 Tetel	×47	82 v 50 24 v 200	000 1	18.76	V 185.2	<u> ۲۰</u>						<u> </u>				
ź		#	Τ				30.9			2.16	2 V 2965	0 339.1	2 4	67 5	5000 1	3.33	£1.Z	21						· · ·				<u>+-</u>
6		1/2	5-13	- 48			25.	27	250 /	35.8	27/ 7		16	39 2	11 V 10.00 V	3.61			,			_						—
6		3					27.1		300 /	345	01 2919	6) 331.1 Votal 3	7 46	10 00	000 ./	1451	· /89.4	9						<u>.</u>	· · · ·			1-
6		#	5-13				20.	96	300 v	34.5	0 V (2963	338.1	7	<u>29 5</u> 49 ×			· 44. • 44.											<u> </u>
5		1	<u>5-13</u>				21.	16	<u>300 V</u>	34.5	V 2937	7		40	5000 4	4.77	× 46.	<u> </u>			· _ ·			 				<u> </u>
5		3	5-13				22.		.300 🗸	343	0 1 292.9	Totol 1	8+5 176	<u>.41 / 20</u> 27 - 5	00.00	<u>23.59</u> 8.73	v 110.	<u>~ ~ </u>										
5		1,	5-14				18.		.275 v	35.1	1 × 296. 1 × 293.	10 <u>338.</u> 59		62 1 1	1 1.51 V	4.00	1 45. V 46.	03							ļ			+-
##		2				ļ	2/.	95		1 24.5	0 V 293. 8 : 297.		× 45	03 v 3 64 v 3	50.00 V	1.1	16	64					1		· · · · · ·		<u></u>	<u>-</u>
4		3	5-1-				22.					7	<u>8+4 176</u>	52 / 12	5000	7.8/	180	19								·		1
<u></u>			5-14	1-48		<u> </u>		.61		1 74	v/ ~ 295 17 ~ 294	13 1			· · · · · ·	5.42	· 45	34		_				<u> </u>				<u>-+-</u>
3	 	- 2				<u> </u>	- 24	5.79		1 24	50 / 293. 88 - 292	27			V	1 5 47	180	03 14		_ 								+-
3		<u>+</u> #	5-14	4-48						\downarrow	50 / 293	(07=1	./7 1 4	4.24	20.00	576		74						1		<u> </u>		\mp
2			5-1	5-48			<u>3/</u>	1.33 192		25	11 - 295.		<u>+</u> +	<u>2.83</u> 4.21	#8.67	444	. 45	21	<u>.</u>	_								<u> </u>
2		3	_	1		1		7.33	. <u>250</u> . 300	/	50 V 192.	93 338	17 U 8+2 17	5.24 6.52 V 1	50.00	/ 7.4.1	5 190	52			οτα							
2			5-1:	5-48		1		4.78	. 275	. 35.	1 298	37 33	2.17 Ľ.S	980 272	50.00	2.20	4	22		G	RAN		<u> </u>					<u></u>
		17	5-1	7-41		<u>+</u>	2	7.20		36.	6/ v 29-	<u>195 - 1</u>		483 <u> </u>		9.17			*		T NO.	FOOTING	PILE N		CONCER	RNING AN	VY UNUS	SUAL
	<u>+</u>	3					2	7.8/ 9.64	300				. <u>12</u> [<u>8</u>] [] [i
										Tet	6 b Doc	Ket 12.2	48 23	18-54/ 2	(117.25)							-						
F=		-+-				1				+														_				
	1	<u> </u>		<u> </u>																								
	+								·													· · ·						
	+	_								1-																		
																				┉╢╼──				-				
 	-1		_														<u> </u>											
11	_	1																										

	· · · · ·							6.0.17.0.40	•
N, PER BLOW		ELEVATION PILE TIP	ELEVATION CUT-OFF	LENGTH	ORIGINAL LENGTH	LENGTH CUT-OFF	LENGTH	C.O. AT 0.40 OF UNIT PRICE	OF UNIT
	· · · ·								
								·	
								· · · · ·	
	<u> </u>						 		
					·				
								· · · · · · · · · · · · · · · · · · ·	
		<u> </u>	· · · · · · · · · · · · · · · · · · ·					· · ·	
						,			1
							· · · · · · · · ·		
<u>.</u>									
<u></u>							••		
-			÷				-		
							<u></u>		
<u> </u>									
			<u> </u>						
				· · · ·			·		•
		· · · · ·							
		[
							<u> </u>	<u> </u>	
							2."		
	·	_							
		· · · ·					<u>.</u>		
				<u> </u>			••		
		VOATION	CONDITIO	N				<u>.</u>	
///////	IN EOU		00.00000000						
UNUS	VAL FOU	REM	ARKS						
UNUS	VAL FOU	REM	ARKS				` _		
UNUS		REM	A R K S					<u></u>	
		REM	· · · · · · · · · · · · · · · · · · ·		· · · · · · · · · · · · · · · · · · ·			<u>/</u> /	
		REM	· · · · · · · · · · · · · · · · · · ·					1) }	
		REM	· · · · · · · · · · · · · · · · · · ·						/
		REM	· · · · · · · · · · · · · · · · · · ·						
		REM	· · · · · · · · · · · · · · · · · · ·						7
		REM	· · · · · · · · · · · · · · · · · · ·						

.

. .

. .

