U.S. DEPARTMENT OF THE INTERIOR U.S. GEOLOGICAL SURVEY

LEVEL II BRIDGE SCOUR ANALYSIS FOR STRUCTURE 124009700800 ON ROUTE SC 97, CROSSING ROCKY CREEK IN CHESTER COUNTY, SOUTH CAROLINA

By Andy W. Caldwell and Michael G. Zalants

Prepared in cooperation with the SOUTH CAROLINA DEPARTMENT OF TRANSPORTATION



Columbia, South Carolina 1994

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UNIT ABBREVIATIONS

cubic foot per second	ft^3/s
feet per second	ft/s
foot	ft
mile	mi
millimeter	mm
square foot	ft ²
square mile	mi ²

OTHER ABBREVIATIONS

downstream	D/S
upstream	U/S
flood plain	f/p
median diameter of bed material	D_{50}
Water-Surface Profile computation model	WSPRO
South Carolina Department of Transportation	SCDOT

In this report, the words "right" and "left" refer to directions that would be reported by an observer facing downstream.

Sea level: In this report, "sea level" refers to the National Geodetic Vertical Datum of 1929-- a geodetic datum derived from a general adjustment of the first-order level nets of the United States and Canada, formerly called Sea Level Datum of 1929.

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Level II bridge scour analysis for structure 124009700800 on Route SC 97, crossing Rocky Creek in Chester County, South Carolina

by Andy W. Caldwell and Michael G. Zalants

This report provides the results of the detailed Level II analysis of scour potential at structure 124009700800 on Route SC 97, crossing Rocky Creek in Chester County, South Carolina (figure 1 in pocket; figures 4-7). The site is located in the Piedmont physiographic province near the town of Great Falls in the southern part of Chester County. The drainage area for the site is 126 mi², and is a predominantly rural drainage basin with little development in recent years. In the vicinity of the study site, the land is covered by moderately thick hardwoods and with an old clear-cut area on the upstream left flood plain.

In the study area, Rocky Creek has a meandering channel with a slope of approximately 0.0019 ft/ft (10.0 ft/mi), an average channel top width of 104 ft and an average channel depth of 12.9 ft. The predominant channel bed material is sand (D_{50} is 1.4 mm) and the channel banks consist of a silty sand (D_{50} is 0.32 mm). In general, the banks have moderate woody vegetative cover and were noted to be relatively stable at the time of the Level I and Level II site visits, July 19, 1990 and September 27 and 28, 1993, respectively.

The Route SC 97 crossing of Rocky Creek is a 413-ft-long, two-lane bridge consisting of one 70-ft and six 53- to 62-ft concrete arch spans, supported by steel and concrete bents with concrete arch abutments. The left and right abutments are not protected by riprap. In this report, the words "right" and "left" refer to directions that would be reported by an observer facing downstream. Additional details describing conditions at the site are included in the Scour Report Summary.

Scour depths were computed using engineering judgement and the general guidelines described in Hydraulic Engineering Circular 18 (Richardson and others, 1993) and the Transportation Research Board Draft Paper, "Evaluating scour at bridges using WSPRO" (Arneson and others, 1992). Scour depths were calculated assuming an infinite depth of erosive material and a homogeneous particle-size distribution. The results of the scour analysis are presented in tables 1 through 5 and a graph of the scour depths is shown on figure 2.

Scour depth calculations indicate that pile tip exposure will occur at bents 1 and 2 for
the 100- and 500-year discharges. Scour caused by the 100-year discharge will
undermine bents 1 and 2 by 7.6 and 0.5 ft, respectively. Scour caused by the 500-year
discharge will undermine bents 1 and 2 by 13.5 and 6.5 ft, respectively.

It should be noted that the SCDOT bridge plan borings (docket number 12.328) show subsurface rock and gravel deposits that could affect the scour depths shown in this study. For more information, see the SCDOT bridge plans in the pocket at the back of the report.

Table 1. --Remaining pile/footing penetration at piers/bents for the 100-year discharge at structure 124009700800 on Route SC 97, crossing Rocky Creek in Chester County, South Carolina

Remaining ⁵ pile/footing penetration (feet)		6.9	6.6	6'9	6.5	-0.5	-7.6
Elevation of scour, USGS datum (feet)		69.1	67.3	64.1	68.0	68.5	61.8
Total ⁴ scour depth (feet)	r second	13.6	13.8	10.4	10.4	14.5	15.1
Ground elevation at pier/bent, USGS datum (feet)	100-year discharge is 16,900 cubic feet per second	82.7	81.1	74.5	78.4	83.0	76.9
Pile tip/ footing elevation, USGS datum (feet)	discharge is 16,9	62.2	57.4	57.2	61.5	0.69	69.4
Pile tip/3 footing elevation, SCDOT datum (feet)	100-year (213.2	208.4	208.2	212.5	220.0	220.4
Station from ² left end of bridge (feet)		53	109	170	240	302	360
Pier/bent ¹ number		9	5	4	٣	2	1

 $^{^1\,\}mathrm{Pier}/\mathrm{bent}$ number corresponds to the South Carolina Department of Transportation (SCDOT) bridge plans.

NOTE: The SCDOT bridge plan borings (docket number 12.328) show subsurface rock and gravel deposits that could reduce the scour depths shown in the above table. For more information, see the SCDOT plans in report pocket.

² Stations are determined from left to right looking downstream.

³ Pile tip/footing elevations obtained from the SCDOT bridge plans. The maximum elevation at each pier/bent for the widened structure is used. No elevation data was received for the arch piers from the SCDOT.

⁴ Total scour depth is the sum of the contraction and pier/bent scour depths.

 $^{^5}$ A negative number signifies undermining of pile tip/footing.

 Table 2. --Remaining pile/footing penetration at piers/bents for the 500-year discharge at structure 124009700800 on Route SC 97, crossing Rocky
 Creek in Chester County, South Carolina

n of Remaining ⁵ ; pile/footing fum penetration (feet)		6.0	3.9	5.8	5.4	-6.5	-13.5
Elevation of scour, USGS datum (feet)		63.1	61.3	63.0	6.99	62.5	55.9
Total ⁴ scour depth (feet)	er second	19.6	19.8	11.5	11.5	20.5	21.0
Ground elevation at pier/bent, USGS datum (feet)	500-year discharge is 24,400 cubic feet per second	82.7	81.1	74.5	78.4	83.0	76.9
Pile tip/ footing elevation, USGS datum (feet)	discharge is 24,	62.2	57.4	57.2	61.5	69.0	69.4
Pile tip/ ³ footing elevation, SCDOT datum (feet)	500-year	213.2	208.4	208.2	212.5	220.0	220.4
Station from ² left end of bridge (feet)		53	109	170	240	302	360
Pier/bent ¹ number		9	\$	4	33	2	

¹ Pier/bent number corresponds to the South Carolina Department of Transportation (SCDOT) bridge plans.

NOTE: The SCDOT bridge plan borings (docket number 12.328) show subsurface rock and gravel deposits that could reduce the scour depths shown in the above table. For more information, see the SCDOT plans in report pocket.

² Stations are determined from left to right looking downstream.

³ Pile tip/footing elevations obtained from the SCDOT bridge plans. The maximum elevation at each pier/bent for the widened structure is used. No elevation data was received for the arch piers from the SCDOT.

⁴ Total scour depth is the sum of the contraction and pier/bent scour depths.

 $^{^5}$ A negative number signifies undermining of pile tip/footing.

Table 3. --Cumulative scour depths at piers/bents for the 100-year discharge at structure 124009700800 on Route SC 97, crossing Rocky Creek in Chester County, South Carolina

Pier/bent ¹ number	Station from ² left end of bridge (feet)	Contraction scour depth (feet)	Pier/bent scour depth without debris (feet)	Total ³ scour depth without debris (feet)
	100-year dischar	ge is 16,900 cubi	ic feet per second	
6	53	6.0	7.6	13.6
5	109	6.0	7.8	13.8
4	170	0^{4}	10.4	10.4
3	240	0^4	10.4	10.4
2	302	7.0	7.5	14.5
1	360	7.0	8.1	15.1

¹ Pier/bent number corresponds to the South Carolina Department of Transportation (SCDOT) bridge plans.

NOTE: The SCDOT bridge plan borings (docket number 12.328) show subsurface rock and gravel deposits that could reduce the scour depths shown in the above table. For more information, see the SCDOT plans in report pocket.

NOTE: The pier and contraction scour equations used in this scour analysis were those recommended in Hydraulic Engineering Circular 18 (Richardson and others, 1993). Scour depths were calculated assuming an infinite depth of erosive material and a homogeneous particle-size distribution.

² Stations are determined from left to right looking downstream.

³ Total scour depth is the sum of the contraction and pier/bent scour depths.

⁴ The calculated contraction scour is a negative value, but was set equal to zero to reflect a more reasonable estimate of scour during peak flood conditions.

Table 4. -- Cumulative scour depths at piers/bents for the 500-year discharge at structure 124009700800 on Route SC 97, crossing Rocky Creek in Chester County, South Carolina

Pier/bent ¹ number	Station from ² left end of bridge (feet)	Contraction scour depth (feet)	Pier/bent scour depth without debris (feet)	Total ³ scour depth without debris (feet)
	500-year discharg	ge is 24,400 cub	ic feet per second	
6	53	10.7	8.9	19.6
5	109	10.7	9.1	19.8
4	170	0^{4}	11.5	11.5
3	240	0^4	11.5	11.5
2	302	11.5	9.0	20.5
1	360	11.5	9.5	21.0

¹ Pier/bent number corresponds to the South Carolina Department of Transportation (SCDOT) bridge plans.

NOTE: The SCDOT bridge plan borings (docket number 12.328) show subsurface rock and gravel deposits that could reduce the scour depths shown in the above table. For more information, see the SCDOT plans in report pocket.

NOTE: The pier and contraction scour equations used in this scour analysis were those recommended in Hydraulic Engineering Circular 18 (Richardson and others, 1993). Scour depths were calculated assuming an infinite depth of erosive material and a homogeneous particle-size distribution.

² Stations are determined from left to right looking downstream.

 $^{^3}$ Total scour depth is the sum of the contraction and pier/bent scour depths.

⁴ The calculated contraction scour is a negative value, but was set equal to zero to reflect a more reasonable estimate of scour during peak flood conditions.

Table 5. -- Abutment scour depths for the 100- and 500-year discharges at structure 124009700800 on Route SC 97, crossing Rocky Creek in Chester County, South Carolina

Recurrence interval for discharge	Discharge (cubic feet per second)	Depth of scour ^{1, 2} at left abutment (feet)	Depth of scour ^{1, 2} at right abutment (feet)
100-year	16,900	7.6	12.5
500-year	24,400	14.7	18.2

 $^{^{1}}$ Abutment scour depths were calculated using the Froehlich (1989) live-bed abutment scour equation, assuming no abutment protection.

² The words "right" and "left" refer to directions that would be reported by an observer facing downstream.

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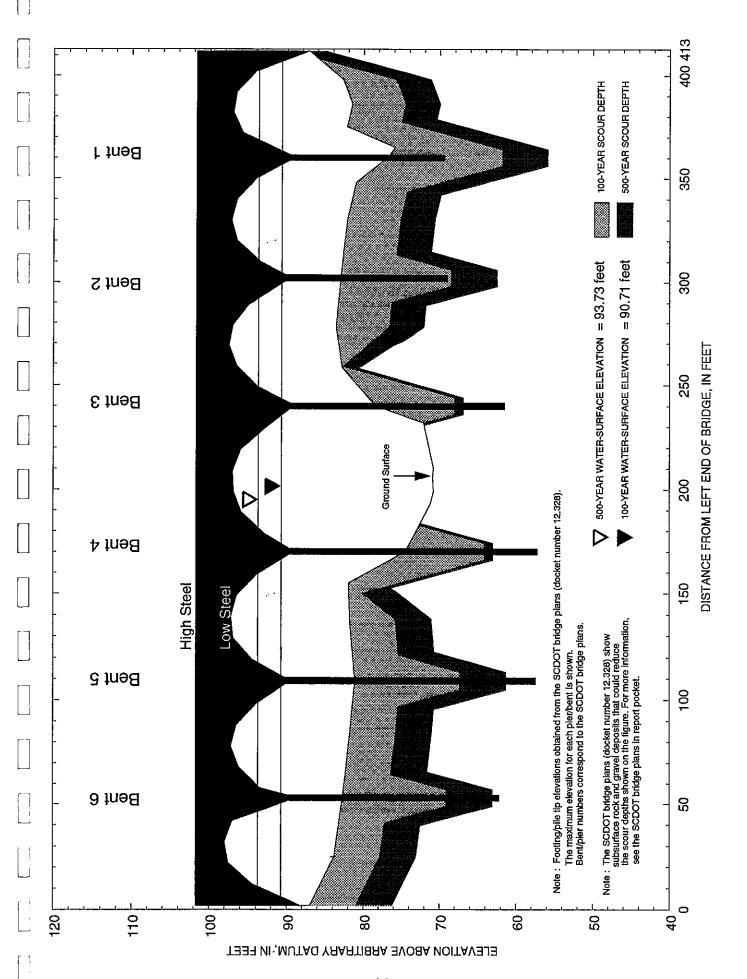


Figure 2.--Total scour depths for the 100- and 500-year discharges at structure 124009700800 on Route SC 97, crossing Rocky Creek in Chester County, South Carolina.

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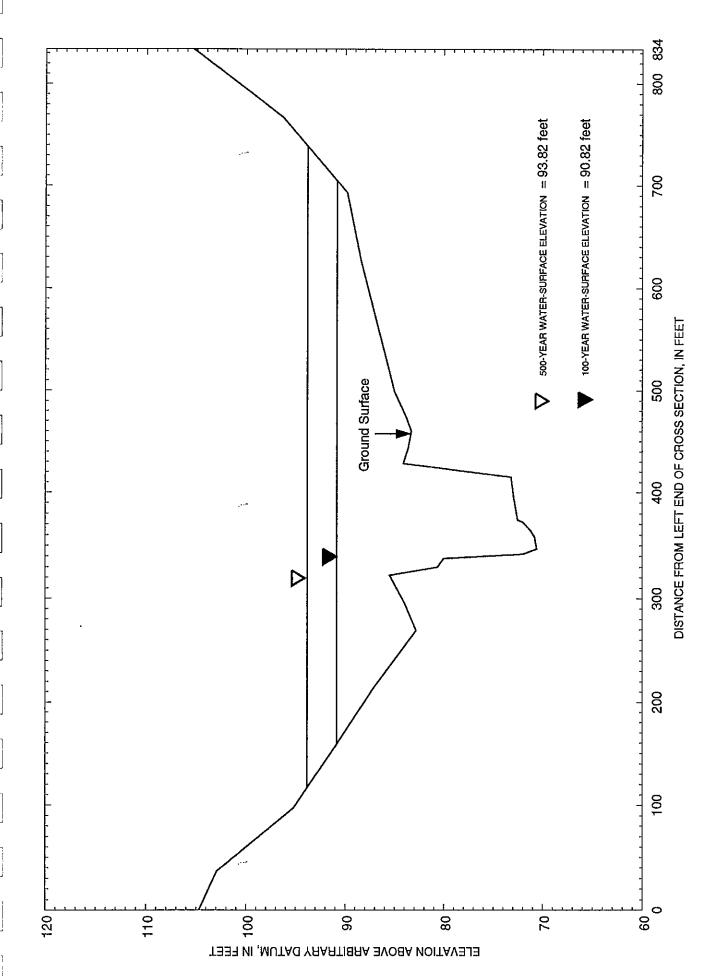


Figure 3.--Approach cross section at structure 124009700800 on Route SC 97, crossing Rocky Creek in Chester County, South Carolina.

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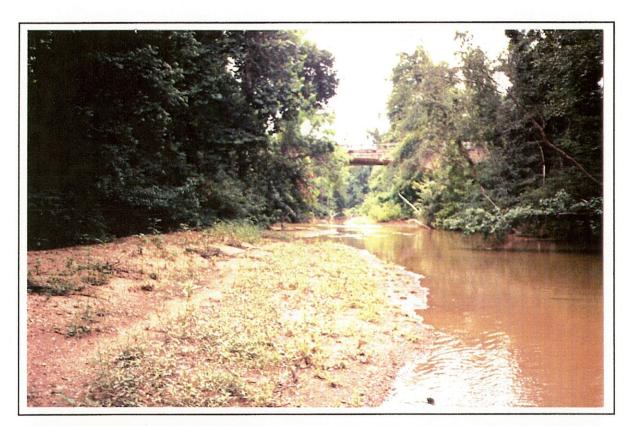


Figure 4.--Structure 124009700800 on Route SC 97, crossing Rocky Creek in Chester County, South Carolina as viewed from upstream (July 19, 1990).



Figure 5.--Upstream channel as viewed from structure 124009700800 on Route SC 97, crossing Rocky Creek in Chester County, South Carolina (July 19, 1990).

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Figure 6.--Structure 124009700800 on Route SC 97, crossing Rocky Creek in Chester County, South Carolina as viewed from downstream (July 19, 1990).



Figure 7.--Downstream channel as viewed from structure 124009700800 on Route SC 97, crossing Rocky Creek in Chester County, South Carolina (July 19, 1990).

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Arcement, G.J., Jr., and Schneider, V.R., 1989, Guide for selecting Manning's roughness coefficients for natural channels and flood plains: U.S. Geological Survey Water-Supply Paper 2339, 38 p. Arneson, L. A., Shearman, J. O., Jones, J. S., 1992, Evaluating scour at bridges using WSPRO: Transportation Research Board Draft Paper, 40 p. Bohman, L. R., 1990, Determination of flood hydrographs for streams in South Carolina: Volume 1. Simulation of flood hydrographs for rural watersheds in South Carolina: U.S. Geological Survey Water-Resources Investigations Report 89-4087, 53 p. Bohman, L. R., 1992, Determination of flood hydrographs for streams in South Carolina: Volume 2. Estimation of peak-discharge frequency, runoff volumes, and flood hydrographs for urban watersheds: U.S. Geological Survey Water-Resources Investigations Report 92-4040, 79 p. Froehlich, D. C., 1989, Local scour at bridge abutments in Ports, M. A., ed., Hydraulic Engineering-Proceedings of the 1989 National Conference on Hydraulic Engineering: New York, American Society of Civil Engineers, p. 13-18. Guimaraes, W. B., and Bohman, L. R., 1991, Techniques for estimating magnitude and frequency of floods in South Carolina, 1988: U.S. Geological Survey Water-Resources Investigation Report, 91-4157, 174 p. Gunter, H.E., Mason, R.R., and Stamey, T.C., 1987, Magnitude and frequency of floods in rural and urban basins in North Carolina: U.S. Geological Survey Water-Resources Investigations Report, 87-4096, 54 p. Laursen, E. M., 1960, Scour at bridge crossings: Journal of the Hydraulics Division, American Society of Civil Engineers, v. 86, no. HY2, p. 39-53. Laursen, E. M., 1963, An analysis of relief bridge scour: Journal of the Hydraulics Division, American Society of Civil Engineers, v. 89, no. HY3, p. 93-118. Richardson, E. V., Harrison, L. J., Richardson, J. R., and Davis, S. R., 1993, Evaluating scour at bridges: Federal Highway Administration Hydraulic Engineering Circular No. 18, Publication FHWA-IP-90-017, 131 p. Richardson, E. V., Simons, D. B., and Julien, P. Y., 1990, Highways in the river environment: Federal Highway Administration Publication FHWA-HI-90-016. Richardson, E. V., Simons, D. B., Karaki, S., Mahmood, K., and Stevens, M. A., 1975, Highways in the river environment: hydraulic and environmental design considerations: Federal Highway Administration. Shearman, J. O., 1990, User's manual for WSPRO--a computer model for water surface profile computations: Federal Highway Administration Publication FHWA-IP-89-027, 187 p. Shearman, J. O., Kirby, W. H., Schneider, V. R., and Flippo, H. N., 1986, Bridge waterways analysis model; research report: Federal Highway Administration Publication FHWA-RD-86-108, 112 p.

U.S. Geological Survey, Interagency Advisory Committee on Water Data, 1982, Guidelines for determining flood flow frequency, Bulletin 17B of the Hydrology Subcommittee, 190 p.

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SCOUR REPORT SUMMARY

		Stream	Rocky Creek
untyCheste	er .	Road SC	97
	Descript	ion of Bridge	
Bridge length	413 ft Bridge wid	th 31 ft	Max span length70 ft
Alignment of br	ridge to road (on curve or	straight) stra	night
Abutment type	vertical	_ Embankment t	ype sloping
Riprap on abutn	nent? no	Date of inspection	n 7-19-1990
Description of abutments.	riprap No riprap was p	placed at the toe o	f the concrete arch
,	on of piers/pile bents Si		o more prore out port the
piles at the dow	vnstream end of the bridged to flood plain according	e. g to USGS topo n	with two 1.0 ft by 1.0 ft steel H- nap? _yes
piles at the dow Is bridge skewe Is bridge locate	vnstream end of the bridged to flood plain according	e. g to USGS topo n no If so, desc	with two 1.0 ft by 1.0 ft steel H- nap? _yes
piles at the down Is bridge skewe Is bridge locate The channel is	vnstream end of the bridged to flood plain according of on a bend in channel?	e. g to USGS topo n no If so, desc	with two 1.0 ft by 1.0 ft steel H- nap? _yes
piles at the down Is bridge skewe Is bridge locate The channel is approximately	vnstream end of the bridged to flood plain according of on a bend in channel? fairly straight through the	ge. g to USGS topo n no If so, desc e bridge but begin	with two 1.0 ft by 1.0 ft steel H- nap? _yes
piles at the down Is bridge skewe Is bridge locate The channel is approximately	vnstream end of the bridged to flood plain according of on a bend in channel? fairly straight through the 150 ft downstream.	ge. g to USGS topo m no	with two 1.0 ft by 1.0 ft steel H- nap? _yes
piles at the down Is bridge skewe Is bridge locate The channel is approximately	vnstream end of the bridged to flood plain according of on a bend in channel? fairly straight through the 150 ft downstream.	ge. g to USGS topo m no	with two 1.0 ft by 1.0 ft steel H- nap? _yes
piles at the down Is bridge skewe Is bridge locate The channel is approximately Debris accumulately	vnstream end of the bridged to flood plain according of on a bend in channel? fairly straight through the 150 ft downstream. Lation on bridge at time of Date of inspection	g to USGS topo n no	with two 1.0 ft by 1.0 ft steel H- nap? yes Angle 8 cribe (mild, moderate, severe) as a mild bend at II site visit: nel Percent of channe tally blocked vertically
piles at the down Is bridge skewe Is bridge locate The channel is approximately Debris accumulately Level I Level II	vinstream end of the bridged to flood plain according of an abend in channel? fairly straight through the 150 ft downstream. I ation on bridge at time of Date of inspection 7-19-1990 9-27-1993	g to USGS topo m no If so, desc e bridge but begin f Level I or Level Percent of chan blocked horizon	with two 1.0 ft by 1.0 ft steel H- nap? yes Angle 8 cribe (mild, moderate, severe) as a mild bend at II site visit: nel Percent of channe tally blocked vertically

Description of Flood Plain

General top	ography _	Typical P	iedmont topo	graphy with rolling l	nills and a relatively
narrow floo	d plain.				
Flood-plai	n condition	ıs at bridg	e site: downs	tream (D/S), upstrear	n (U/S)
Date of ins	pection _9	-27-1993			
D/S left:	Moderate	ely thick h	nardwoods		
D/S right:	Moderate	ly thick h	ardwoods		
U/S left:	Old clear	cut area w	ith potential	for rapid growth of t	he young vegetation
U/S right:	Moderate	ely thick h	ardwoods		
		<u>De</u>	escription o	of Channel	
Average top	width _	104	ft	Average de	pth 12.9 ft
Predomina	nt bed mat	e rial <u>s</u>a	ınd	Bank materia	al silty sand
Stream typ:	e (straight,	meanderi	ing, braided, s	swampy, channelized) meandering
			_	, ,	
Vegetative (cover on ch	annel ban	ıks near b ri dg	ge: Date of inspectio	on 9-27-1993
D/S left:	Moderate	ely thick v	voody vegeta	tion	
D/S right:	Moderate	ely thick v	voody vegeta	tion	
U/S left:	Light wo	ody veget	ation		
U/S right:	Moderate	ly thick w	voody vegeta	tion	
Do banks a	ppear stabi	le? <u>yes</u>	_ If not,	describe location and	l type of instability and
date of obs	ervation.	Overall,	the banks ap	pear stable with som	e localized bank
failure not	ed on the c	lownstrea	m left bank a	t the impact point du	iring the Level I
inspection	n on 7-19-19	990.			
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	·				
Deccribe an	n obstruct	ione in ch	annal and da	te of observation.	None observed
	y oosiineii	ons in th	**************************************	c of ooser our ion.	
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<u>Hydrology</u>

Percentage of drainage area in physiographic provinces: Physiographic province Percent of drainage area Piedmont (high-flow area) 100 Is drainage area considered rural or urban? rural Describe any significant urbanization and potential for development. There is no significant urbanization and little potential for development in the Rocky Creek drainage basin. Is there a USGS gage on the stream of interest? yes USGS gage description Rocky Creek at Great Falls, SC USGS gage number 02147500
Piedmont (high-flow area) Is drainage area considered rural or urban? rural Describe any significant urbanization and potential for development. There is no significant urbanization and little potential for development in the Rocky Creek drainage basin. Is there a USGS gage on the stream of interest? yes USGS gage description Rocky Creek at Great Falls, SC
Is drainage area considered rural or urban? rural Describe any significant urbanization and potential for development. There is no significant urbanization and little potential for development in the Rocky Creek drainage basin. Is there a USGS gage on the stream of interest? yes USGS gage description Rocky Creek at Great Falls, SC
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Is there a USGS gage on the stream of interest? yes USGS gage description Rocky Creek at Great Falls, SC
USGS gage description Rocky Creek at Great Falls, SC
USGS gage description Rocky Creek at Great Falls, SC
USGS gage number 02147500
Gage drainage area 194 mi ²
Is there a lake/pond that will significantly affect hydrology/hydraulics?no
If so, describe
1) 50) 46567 666
Calculated Discharges
_
Q100 $16,900$ ft^3/s Q500 $24,400$ ft^3/s
Method used to determine discharges Because the basin is in the high-flow region of S.C.
the 100- and 500-year discharges were determined by using the N.C rural regression
equations and methods described in USGS Bulletin 17B. The rural regression discharges
were then weighted with the gage data by methods described on pages 18 through 20 of
WRIR 87-4096 (Gunter, Mason, and Stamey, 1987).

Datum for WSPF	RO analysis (USGS sı	rvey, sea level, SCDOT	plans) USGS survey
Datum tie betwe	en USGS survey and	SCDOT plans Ad	d 151.0 ft to the USGS
survey to obtain	n the SCDOT plan's o	latum (docket number 1	12.328).
Description of re	eference marks used t	o determine USGS datu	m. RM 1 is a chiseled
square on the u	pstream left headwal	of the Route SC 97 brid	dge with a surveyed
elevation of 99.9	94 ft. RM 2 is a chisele	d square on the downst	ream right headwall of the
Route SC 97 bri	dge with an assumed	l elevation of 100.00 ft.	
	Cross Sections I	Jsed in WSPRO Anal	veie
	Cross occions c	oscu iii worko Aliai	y 515
Cross section ID	Section Reference Distance (SRD) in feet	**How cross section was developed	Comments
EXIT	413	2	Exit cross section
FULV	0	2	Full valley cross section
BRIDG	0	1	Upstream bridge face
APPR	444	2	Approach cross section
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Description of data and assumptions used in developing WSPRO model.

Rocky Creek has a relatively uniform flood plain width in the study area, with no downstream natural or man-made contractions of flow that cause significant backwater at the Route SC 97 crossing. Therefore, it was assumed that slope-conveyance methodology would be adequate for estimating the starting water-surface elevation for the water-surface profile computations.

For this study, the WSPRO model requires, as a minimum, an exit section one bridge width downstream of the bridge, a full-valley section at the downstream face of the bridge, the bridge section, and an approach section one bridge width upstream of the bridge. Cross sections at the upstream and downstream faces of the bridge were directly surveyed and the more constricted (upstream) bridge face was used in the WSPRO model. The section reference distance (SRD) at the downstream face of the bridge was set to zero. Surveys of the exit and approach channels (located at 544 ft downstream of the upstream bridge face and 330 ft upstream of the upstream bridge face, respectively) were superimposed on the flood plain survey taken along the upstream toe-of-fill of Route SC 97. These cross sections were shifted by the channel slope to the appropriate SRD to represent the exit, full-valley, and approach cross sections required by the WSPRO model.

Bridge Hydraulics

Average embankment elevation 99.7 f

Average low steel elevation 97.0 ft

100-year discharge 16,900 ft³/s

Water-surface elevation at D/S bridge face 90.71 ft

Area of flow at D/S bridge face 4,162 ft²

Average velocity in bridge opening 4.06 ft/s

Maximum WSPRO tube velocity at bridge 6.49 ft/s

Water-surface elevation at Approach section with bridge 90.82 ft

Water-surface elevation at Approach section without bridge 90.84 ft

Amount of backwater caused by bridge 0^* ft

500-year discharge 24,400 ft³/s

Water-surface elevation at D/S bridge face 93.73 ft

Area of flow at D/S bridge face 5,128 ft²

Average velocity in bridge opening 4.76 ft/s

Maximum WSPRO tube velocity at bridge 7.86 ft/s

Water-surface elevation at Approach section with bridge 93.82 ft

Water-surface elevation at Approach section without bridge 93.80 ft

Amount of backwater caused by bridge 0.02 ft

*Backwater for the 100- year discharge is -0.02 ft. Because negative backwater is unlikely, it was set to zero.

Scour

Describe any special assumptions or considerations made in bridge scour analysis.

Scour depths were computed using engineering judgement and the general guidelines described in Hydraulic Engineering Circular 18 (Richardson and others, 1993) and the Transportation Research Board Draft Paper, "Evaluating scour at bridges using WSPRO" (Arneson and others, 1992). Scour depths were calculated assuming an infinite depth of erosive material and a homogeneous particle-size distribution. The results of the scour analysis are presented in tables 1 through 5 and a graph of the scour depths is shown on figure 2.

The local pier scour was determined using the Colorado State University pier scour equation (Richardson and others, 1993). Bents 5 and 6 are located on the left overbank and were analyzed using the maximum left overbank WSPRO tube velocity and the depth of flow at each bent. Bents 1 and 2 are located on the right overbank and were analyzed using the maximum right overbank WSPRO tube velocity and the depth of flow at each bent. Bents 3 and 4 are located in the channel and were analyzed using 90 percent of the maximum WSPRO tube velocity and the maximum depth within the channel at the bridge. The maximum depth within the channel was used to account for possible changes in the thalweg during a flood event.

The left and right overbanks at the bridge were analyzed for contraction scour using Laursen's clear-water contraction scour equation (Richardson and others, 1993). The channel contraction scour was analyzed using Laursen's modified live-bed contraction scour equation (Richardson and others, 1993).

The live-bed contraction scour equation indicates the deposition of sediment in the channel at the bridge during the 100- and 500-year floods. (See negative scour values determined in scour calculations included at the end of the report). However, it seems unreasonable to expect sediment deposition at the bridge during peak flood conditions. Therefore, the negative scour values were set equal to zero as reflected in tables 1 through 4 and figure 2.

The abutments are not protected by riprap, therefore abutment scour was calculated
using the Froehlich (1989) live-bed abutment scour equation.
It should be noted that the SCDOT bridge plan borings (docket number 12.328) show
subsurface rock and gravel deposits that could affect the scour depths shown in this study.
For more information, see the SCDOT bridge plans in the pocket at the back of the report.
It should also be noted that the old arch piers for the original structure could possibly
have footings and/or pile groups. However, there was no information available from the
SCDOT for these original substructures.
, me
$oldsymbol{\cdot}$
·

WSPRO INPUT FILE

```
T1
             Structure #124009700800
                                                   (413 ft. bridge)
T2
             Rocky Creek at SC 97
                                              file: rocky.sc97
Т3
             Chester County, South Carolina
                                                    AWC
                                                          September 1994
*
             Q100
                        0500
Q
             16900
                        24400
SK
             .0019
                       .0019
             Survey data for the EXIT cross section was taken at
             the U/S toe-of-fill. The channel survey for the EXIT
             cross section was taken at 544 ft D/S of U/S bridge
             face and then superimposed onto the toe-of-fill survey
             of the flood plain. The distance is determined from the
             Level II survey of 9-27-1993.
TX
     SURV1
              0
                   .0019
GR
                0
                   103.9
                            37
                                         97
                                102.1
                                             94.4
                                                     159
                                                          90.0
                                                                 214
                                                                       86.3
GR
              269
                   82.1
                           327
                                83.4
                                        335 80.4
                                                          73.6
                                                     344
                                                                 360
                                                                       72.1
GR
              373
                   71.5
                           385
                                71.3
                                        402
                                             71.5
                                                                       77.2
                                                     416
                                                          72.0
                                                                 425
GR
              430
                   83:1
                           464
                                83.5
                                        627
                                             87.6
                                                     693
                                                          89.0
                                                                 767
                                                                       95.5
GR
              834
                   104.6
*
XS
     EXIT
             -413
GT
N
             0.15
                    0.045
                             0.15
SA
                 327
                          430
PΧ
*
XS
     FULV
             0
GT
PΧ
*
             U/S Face of Bridge
BR
              0 97.0 8
     BRIDG
GR
              1.9
                   88.2
                           2 86.9
                                      25 83.8
                                                 53 82.7
                                                             82
                                                                 81.8
GR
              109
                   81.1
                           138
                                81.8
                                       155
                                             81.9
                                                    170
                                                          74.5
                                                                 193
                                                                      71.2
GR
              199
                   70.8
                           206
                                70.9
                                        210
                                             70.8
                                                    215
                                                          71.1
                                                                 232
                                                                       72.1
GR.
              240
                   78.4
                           258
                                82.8
                                        278
                                             83.6
                                                                 331
                                                    302
                                                          83.0
                                                                       82.1
GR
              348
                   81.0
                                76.9
                                             75.9
                           360
                                        365
                                                    375
                                                          82.2
                                                                 386
GR
              398
                   82.7
                           411.9 87.1
                                               87.2
                                          412
                                                      402 94.1
GR
              392
                   96.6
                                97.0
                                        372
                           382
                                             95.8
                                                    361
                                                         89.5
                                                                 359
                                                                       89.5
GR
              350
                   94.0
                           340
                                96.5
                                        330
                                             97.2
                                                    320
                                                          96.5
                                                                 310
                                                                       93.5
GR
              303
                   90.0
                           301
                                90.0
                                        289
                                             95.0
                                                    279
                                                          97.0
                                                                 269
                                                                       97.5
GR
              259
                   96.5
                                93.8
                                             89.5
                           249
                                        241
                                                    239
                                                          89.5
                                                                 229
                                                                      93.0
GR
              219
                   96.0
                           209
                                97.0
                                       199
                                             96.9
                                                    189
                                                          95.9
                                                                 179
                                                                      93.0
GR
              171
                   89.5
                           169
                                89.5
                                       159
                                             93.9
                                                    149
                                                          96.5
                                                                 139
                                                                      97.2
GR
              129
                   96.9
                           119
                                94.5
                                       110
                                             90.0
                                                    108
                                                          90.0
                                                                 98 94.0
ÇR
              88
                  96.5
                          78 97.2
                                     68
                                         96.2
                                                 58
                                                     93.5
                                                             54
                                                                 89.5
GR
              52
                  89.5
                          42 97.0
                                     32
                                         98.0
                                                 22
                                                     97.5
                                                             12
                                                                 94.5
GR
              1.9
                  88.2
Ν
             0.05
                    0.045
                             0.05
SA
                 155
                          258
```

WSPRO INPUT FILE -- Continued

```
2 31 2 99.7
CD
PW 1
             74.5 3.25
                           76.9 3.25
                                        76.9 6.5
                                                     78.4 6.5
                                                                 78.4 9.75
PW
              81.1 9.75
                           81.1
                                 13.0
                                        82.7 13.0
                                                     82.7 16.25 83.0 16.25
PW
              83.0 19.5
                           89.5 19.5
                                        89.5 6.5
                                                     90.0 6.5
                                                                 90.0 0
PX
            Data for the APPROACH cross section was obtained by superimposing
            a survey of the U/S channel section at 330 ft U/S of the
            U/S bridge face on the U/S toe-of-fill flood plain survey
            of 9-28-1993.
ΥT
     SURV2
             0 .0019
GR
                0 103.9
                           37
                               102.1
                                       97
                                           94.4
                                                  159
                                                        90.0
                                                               214
                                                                    86.3
                                           84.7
GR
             269
                   82.0
                          296
                               83.2
                                      322
                                                  330
                                                        79.8
                                                               338
                                                                    79.2
GR
             342
                   71.2
                          347
                               69.8
                                      358
                                           70.0
                                                  364
                                                        70.4
                                                               372
                                                                    71.2
GR
             374
                   71.7
                          394
                               72.1
                                      415
                                           72.4
                                                  428
                                                        83.3
                                                               443
                                                                    82.8
GR
             460
                   82.5
                          474
                               83.0
                                      499 84.2
                                                  627
                                                        87.6
                                                               693
                                                                    89.0
GR
             767
                   95.5
                               104.6
                          834
*
AS
     APPR
            444
GT
N
            0.15
                    0.045
                            0.15
                322
SA
                         428
BP
            167
PX
*
*
*
HP 1 BRIDG
             90.71 0
                        90.71
             90.76
HP 2 BRIDG
                    0
                        90.76
                               16900
HP 1 APPR
             90.82
                    0
                        90.82
HP 2 APPR
             90.82
                    0
                        90.82
                               16900
HP 1 BRIDG
             93.73
                    0
                        93.73
HP 2 BRIDG
             93.79
                    0
                       93.79
                              24400
HP 1 APPR
             93.82
                    0
                       93.82
HP 2 APPR
             93.82
                    0
                       93.82
                              24400
EΧ
ER
```

WSPRO OUTPUT

					VEY
MODE	L FOR WALL	ER-SURFACE	PROFILE C	OMPUTATIONS	
Structure #1	24009700800		(413 ft. b	ridae)	
Rocky Creek	at SC 97	fil	e: rocky.sc	97	
Chester Coun	ty, South Ca	rolina	AWC Sep		
SECTION, PROP	ERTIES: ISE	EQ = 3; S	ECID = BRID	G; SRD =	0.
SA# ARE	A K	TOPW W	ETP ALPH	LEW REW	, QCR
		138	169		20688
					40303
					22871
416	2 589102	361	466 1.20	2 412	73265
					VEY
MODE	L FOR WATE	R-SURFACE	PROFILE CO	OMPUTATIONS	
Structure #1	24009700800		(413 ft h	ridae)	
Rocky Creek	at SC 97	file	e: rockv.sc	97	
Chester Count	y, South Ca	rolina	AWC Sept	tember 1994	
*** RUN DATI	E & TIME: 10	-04-94 07	:40		
TY DISTRIBUT:	ION: ISEQ =	3; SECI	D = BRIDG;	SRD =	0.
SEL LEW	PEW A	PEA	к о	VET.	
				4.04	
			112.3		
322.7		2/1			
			5 233.8		
2.62	3.26			3 226.2 L 3.74	
		3.2	3 3.63	L 3.74	
162.0	176.4	3.23	3 3.63 191.9	198.7	205.3
162.0 212.9	176.4 9 145.2	3.23 184.7 136.0	3.63 191.9 0 133.0	L 3.74	205.3
162.0 212.9 3.9	176.4 9 145.2 7 5.82	3.2: 184.7 136.: 6.2:	3 3.63 191.9 0 133.0 1 6.35	198.7 198.7 0 130.2 6 6.49	205.3
162.0 212.9 3.9	176.4 9 145.2 7 5.82	3.23 184.7 136.0 6.23	3 3.65 191.9 0 133.0 1 6.35	198.7 198.7 130.2 6.49	205.3
162.0 212.9 3.9 205.3 130.8	176.4 7 145.2 7 5.82 212.0 3 133.2	3.23 184.7 136.0 6.23 218.8 139.3	3 3.65 191.9 0 133.0 1 6.35 226.1 2 154.6	198.7 198.7 130.2 6.49 234.6 5 265.0	205.3
162.0 212.9 3.9 205.3 130.8	176.4 7 145.2 7 5.82 212.0 3 133.2	3.23 184.7 136.0 6.23 218.8 139.3	3 3.65 191.9 0 133.0 1 6.35 226.1 2 154.6	198.7 198.7 130.2 6.49	205.3
162.0 212.9 3.9 205.3 130.8 6.46	176.4 7 145.2 7 5.82 212.0 3 133.2 6.34	3.23 184.7 136.0 6.23 218.8 139.3 6.0	3 3.65 191.9 0 133.0 1 6.35 226.1 2 154.6 7 5.46	198.7 198.7 130.2 6.49 234.6 265.0 3.19	205.3 259.6
162.0 212.9 3.9 205.3 130.8 6.46	176.4 7 145.2 7 5.82 212.0 3 133.2 6.34	3.23 184.7 136.0 6.23 218.8 139.3 6.0	3 3.63 191.9 0 133.0 1 6.35 226.1 2 154.6 7 5.46	198.7 198.7 130.2 6.49 234.6 265.0 3.19	205.3 259.6 412.0
162.0 212.9 3.9 205.3 130.8 6.46	176.4 7 145.2 7 5.82 212.0 3 133.2 6.34	3.23 184.7 136.0 6.23 218.8 139.3 6.0	3 3.63 191.9 0 133.0 1 6.35 226.1 2 154.6 7 5.46	198.7 198.7 130.2 6.49 234.6 265.0 3.19	205.3 259.6 412.0
	Structure #1: Rocky Creek Chester Coun *** RUN DAT: SECTION PROP SA# ARE: 1 122: 2 163: 3 130: 416: FEDERAL I MODE: Structure #1: Rocky Creek a Chester Count *** RUN DATE TY DISTRIBUT: SEL LEW .76 1.9 1.9	MODEL FOR WATE Structure #124009700800 Rocky Creek at SC 97 Chester County, South Ca *** RUN DATE & TIME: 10 SECTION, PROPERTIES: ISE SA# AREA K 1 1223 136439 2 1637 304549 3 1302 148114 4162 589102 FEDERAL HIGHWAY ADMI MODEL FOR WATE Structure #124009700800 Rocky Creek at SC 97 Chester County, South Ca *** RUN DATE & TIME: 10 TY DISTRIBUTION: ISEQ = SEL LEW REW A .76 1.9 412.0 418	Structure #124009700800 Rocky Creek at SC 97 fil Chester County, South Carolina *** RUN DATE & TIME: 10-04-94 07 SECTION, PROPERTIES: ISEQ = 3; S SA# AREA K TOPW W 1 1223 136439 138 2 1637 304549 87 3 1302 148114 136 4162 589102 361 FEDERAL HIGHWAY ADMINISTRATION MODEL FOR WATER-SURFACE Structure #124009700800 Rocky Creek at SC 97 file Chester County, South Carolina *** RUN DATE & TIME: 10-04-94 07 TY DISTRIBUTION: ISEQ = 3; SECIONESEL LEW REW AREA .76 1.9 412.0 4180.1 591613	### MODEL FOR WATER-SURFACE PROFILE CONSTRUCTION ### 124009700800 (413 ft. b) ### Rocky Creek at SC 97 file: rocky.sc ### RUN DATE & TIME: 10-04-94 07:40 ### RUN DATE & TIME: 15-04-94 07:40 ### SECTION, PROPERTIES: ISEQ = 3; SECID = BRIDGE ### AREA	Structure #124009700800 (413 ft. bridge) Rocky Creek at SC 97 file: rocky.sc97 Chester County, South Carolina AWC September 1994 *** RUN DATE & TIME: 10-04-94 07:40 SECTION, PROPERTIES: ISEQ = 3; SECID = BRIDG; SRD = SA# AREA K TOPW WETP ALPH LEW REW 1 1223 136439 138 169 2 1637 304549 87 123 3 1302 148114 136 175 4162 589102 361 466 1.20 2 412 FEDERAL HIGHWAY ADMINISTRATION - U. S. GEOLOGICAL SUR MODEL FOR WATER-SURFACE PROFILE COMPUTATIONS Structure #124009700800 (413 ft. bridge) Rocky Creek at SC 97 file: rocky.sc97 Chester County, South Carolina AWC September 1994 *** RUN DATE & TIME: 10-04-94 07:40 TY DISTRIBUTION: ISEQ = 3; SECID = BRIDG; SRD = SEL LEW REW AREA K Q VEL

WSPRO OUTPUT -- Continued

	Rocky Cr Chester	reek at S County,	9700800 C 97 South Car TIME: 10-	fi colina	le: rock AWC	y.sc97	idge) 7 ember :	1994	
CROSS-			Es: ISEQ			APPR ;	: SRD	=	444.
WSEL	2	777 1723	K 21849 343655 28576	163 106	117		LEW	REW	QCR 9632 39436 12918
90.82		3627	394080	545	556 2.	94	159	704	30949
	Structur Rocky Cr Chester	e #12400 eek at S County,	OR WATER 9700800 C 97 South Car TIME: 10-	fi colina	(413 f le: rock AWC	t. bri y.sc97	idge)		
₩	<i>I</i> SEL	LEW	ISEQ = REW AF 4.1 3627	REA	K	Q	VEL		14.
STA. A(I) V(I)		704.9		137	. 4	95.2		89.5	
STA.	354.7	88.7 9.52	9.1 87.5 9.66	86	.1	86.4		88.9	377.4
A(I) V(I)		-			222		207 1		402.2
A(I) V(I) STA. A(I)	377.4	38 87.9 9.61	2.2 87.5 9.66	387.1 88 9.	392.0 .0 60	90.5 9.33	397.1	91.3 9.26	
A(I) V(I) K STA.		38 87.9 9.61	2.2 87.5 9.66 7.3 94.9 8.90	88 9. 412.7	.0 60 419.6	90.5 9.33	466.7	91.3 9.26	704.1

WSPRO OUTPUT -- Continued

WSPRO V04209	4	DERAL H	IGHWAY ADMI FOR WATE						
		MODEL		NISTRATI	CON - U	. S. GE	OLOGICA	AL SUR	VEY
	OL	HODBH	TOR WATE	M 1M UG-M	JE PRO	стпр С	OMPUTAT	LIONS	
	struct	ure #12	4009700800		(41)	3 ft. b	ridae)		
	Rocky	Creek at	4009700800 t SC 97	f	Eile: r	ocky.sc	97		
	Cheste	r County	y, South Ca	rolina	AW	C Sep	tember	1994	
			& TIME: 10						
CRO	DSS-SECTIO	N PROPE	RTIES: ISE	Q = 3;	SECID	= BRID	G; SRI) =	0.
W:	SEL SA#	AREA	K	TOPW	WETP	ALPH	LEW	REW	QC
	1		191443					10111	3436
	2	1857		58					5962
	3		199780						3742
93	.73	5127	714132	277	561	1.11	2	412	
		. *							
WSPRO	FE	DERAL HI	GHWAY ADMI	NTSTRATT	ON - 11	S. GEO	ᡝ᠘ᢙᡟᢈ᠈	ו. פווסי	/FV
V042094			FOR WATE						A C I
								10110	
	Struct	ure #124	1009700800		(413	ft. b	ridae)		
	Rocky	Creek at	SC 97	£	ile: ro	ckv.sc	97 ~		
	Cheste	r County	, South Ca	rolina	AWO	. Sept	ember	1994	
	*** R'	UN DATE	& TIME: 10	-04-94	07:40	-			
VET	הרבע אדפי.	אדיתיו זמד מיתי	ON: ISEQ =	2. 00	CTD - I	DTDG.	CDD		0
V 151	OCIII DIS	INIDOIIC	M: ISEQ =); SE	CID = 1	SKIDG;	SKD =		0.
	WSEL		REW A	REA .	K	Q	VEL	-	
	93.79	1.9	412.0 514	3.7 715	845.	24400.	4.74		
		0	43 6	74.0					
Z COUX	1			74 9	~ ~ ~	- ^	1000		4.40.5
						5.8		0.40 m	147.5
A(I)		338.0	340.2	24	9.7	341.8	3	249.7	
		338.0	3.59	24	9.7	341.8	3	249.7	
A(I) V(I)		338.0 3.61	3.59	24 4	9.7 .89	341.8 3.57	3	249.7 4.89	
A(I) V(I) K STA.		338.0 3.61	3.59	24 4 183.5	9.7 .89 191	341.8 3.57	198.3	249.7 4.89	205.3
A(I) V(I) X STA. A(I)		338.0 3.61 .5 283.2	3.59 169.2 256.3	24 4 183.5	9.7 .89 191 9.7	341.8 3.57 3	198.3	249.7 4.89	205.3
A(I) V(I)		338.0 3.61 .5 283.2	3.59	24 4 183.5	9.7 .89 191 9.7	341.8 3.57 3	198.3	249.7 4.89	205.3
A(I) V(I) STA. A(I) V(I)	147	338.0 3.61 .5 283.2 4:31	3.59 169.2 256.3 4.76	24 4 183.5 16 7	9.7 .89 191 9.7 .19	341.8 3.57 3 157.6 7.74	198.3	249.7 4.89 159.3 7.66	205.3
V(I) X STA. A(I) V(I) X STA.	147	338.0 3.61 .5 283.2 4:31	3.59 169.2 256.3 4.76	24 4 183.5 16 7 219.5	9.7 .89 191 9.7 .19	341.8 3.57 3 157.6 7.74	198.3 5 1 250.7	249.7 4.89 159.3 7.66	205.3
A(I) V(I) X STA. A(I) V(I) X STA. A(I)	147 205	338.0 3.61 .5 283.2 4:31 .3 155.3	3.59 169.2 256.3 4.76 212.2 163.2	24 4 183.5 16 7 219.5	9.7 .89 191 9.7 .19 227 9.7	341.8 3.57 3 157.6 7.74 7.7	198.3 5 250.7	249.7 4.89 159.3 7.66	205.3
A(I) V(I) K STA. A(I) V(I) K STA.	147 205	338.0 3.61 .5 283.2 4:31 .3 155.3	3.59 169.2 256.3 4.76	24 4 183.5 16 7 219.5	9.7 .89 191 9.7 .19 227 9.7	341.8 3.57 3 157.6 7.74 7.7	198.3 5 250.7	249.7 4.89 159.3 7.66	205.3
A(I) V(I) X STA. A(I) V(I) X STA. A(I) V(I)	147 205	338.0 3.61 .5 283.2 4:31 .3 155.3 7.86	3.59 169.2 256.3 4.76 212.2 163.2 7.47	24 4 183.5 16 7 219.5	9.7 .89 191 9.7 .19 227 9.7	341.8 3.57 3 157.6 7.74 7.7 338.2 3.61	198.3 5 4 250.7	249.7 4.89 159.3 7.66 243.1 5.02	205.3
A(I) V(I) X STA. A(I) V(I) X STA. A(I) V(I)	147 205	338.0 3.61 .5 283.2 4:31 .3 155.3 7.86	3.59 169.2 256.3 4.76 212.2 163.2 7.47 312.0	24 4 183.5 16 7 219.5 17 6	9.7 .89 191 9.7 .19 227 9.7 .79	341.8 3.57 3 157.6 7.74 7.7 338.2 3.61	198.3 5 250.7 378.1	249.7 4.89 159.3 7.66 243.1 5.02	205.3
A(I) V(I) STA. A(I) V(I) STA. A(I) V(I)	147 205	338.0 3.61 .5 283.2 4:31 .3 155.3 7.86	3.59 169.2 256.3 4.76 212.2 163.2 7.47	24 4 183.5 16 7 219.5 17 6 334.1	9.7 .89 191 9.7 .19 227 9.7 .79 355	341.8 3.57 3 157.6 7.74 7.7 338.2 3.61	198.3 250.7 378.1	249.7 4.89 159.3 7.66 243.1 5.02	205.3

WSPRO OUTPUT -- Continued

WSPRO V042094	FEDERAL MODE	HIGHWAY ADMIN L FOR WATER	NISTRATION R-SURFACE	- U. S. GEO PROFILE CO	LOGICAL SUR MPUTATIONS	VEY
CROSS	Chester Coun	24009700800 at SC 97 ty, South Car E & TIME: 10- ERTIES: ISEQ	rolina -04-94 07:	AWC Sept 40	ember 1994	444.
WSEL 93.82	SA# ARE 1 132 2 204 3 200 537	A K 8 45790 1 455708 6 69138 6 570637	205 2	05	LEW REW	19186
	FEDERAL					
V042094	MODE Structure #1 Rocky Creek Chester Coun	L FOR WATER	R-SURFACE file	PROFILE CO (413 ft. br : rocky.sc9 AWC Sept	MPUTATIONS idge) 7	
	ITY DISTRIBUT WSEL LEW 3.82 117.1	rew ar	REA 1	Κ. Ο	VEL	14.
A(I)	117.1 915. 1.3	9 495.2	186.0	128.9	119.5	
	353.1 115. 10.5		112.2	114.4	114.5	
X STA. A(I) V(I)	115.	384.1 3 114.7 3 10.64	115.4	116.4	119.3	406.5
X STA. A(I) V(I)	406.5 122. 9.9	412.4 4 142.4 7 8.56	379.3	737.5	997.9	738.3
				-		

WSPRO OUTPUT -- Continued

				,		Contin	ucu		
WSPRO V042094	FEDE:	RAL HIGHW MODEL FO	VAY ADMI OR WATE	NISTRA R-SURI	ATION - FACE I	- U.S. O PROFILE	EOLOGICA COMPUTAT	L SURVEY IONS	
S	tructur	e #124009	700800			(413 ft.	bridge)		
R	ocky Cr	eek at so	97		file	rocky.s	c97		
C		County, S					ptember	1994	
	*** RUN	DATE & T	CIME: 10	-04-94	1 07:4	10			
XSID:CODE	SRDL	· LEW	AREA	VHD	HF	EGL	CRWS	0	WSEL
SRD	FLEN	REW	K	ALPH				VEL	***************************************
EXIT :XS	*****	160	3544	1 06	****	90.20	82.58	16900	89.13
	*****						0.57		03.17
FULV :FV	413	160					*****		89.94
0	413	704	388606	3.01	0.00	0.02	0.57	4.75	
<>< <the "normal"="" (unconstricted)="" above="" flow="" reflect="" results="">>>></the>									
APPR :AS	444	159	3637				*****	16900	90.84
444			395057			0.00		4.65	
<<	<< <the a<="" td=""><td>ABOVE RES</td><td>ULTS RE</td><td>FLECT</td><td>"NORMA</td><td>L" (UNCO</td><td>NSTRICTE</td><td>D) FLOW>:</td><td>>>></td></the>	ABOVE RES	ULTS RE	FLECT	"NORMA	L" (UNCO	NSTRICTE	D) FLOW>:	>>>
	<<< <ri< td=""><td>SULTS RE</td><td>FLECTIN</td><td>G THE</td><td>CONSTR</td><td>RICTED FL</td><td>OW FOLLO</td><td>W>>>></td><td></td></ri<>	SULTS RE	FLECTIN	G THE	CONSTR	RICTED FL	OW FOLLO	W>>>>	
XSID:CODE	SRDL	LEW	AREA	VHD	HF	EGL	CRWS	Q	WSEL
SRD	FLEN	REW	K	ALPH	НО	ERR	FR#		
BRIDG:BR	413	2	41.62	0.26	0.77	90.97	84.79	16900	90.71
0	413						0.21		30.72
түрк р	PCD FLOW	ı c	P/A	LSE	יז. אָד	EN XLA	B XRAB		
2.		0 992							
XSID:CODE	SRDL	LEW	AREA	VHD	HF	EGL	CRWS	Q	WSEL
SRD	FLEN	REW	K	ALPH	НО	ERR	FR#	VEL	
APPR :AS	413	159	3630	0.99	0.79	91.82	83.82	16900	90.82
444		704				0.00			30.02
M(G)	M(K)	ĸQ	XLKQ	VOV	.Q 0	जिल्ला			
0.248		394682.			-	0.07			
			2,11	504		0.07			

WSPRO OUTPUT --Continued

WSPRO V042094	FEDER M						SEOLOGICA COMPUTAT	L SURVEY		
S R	tructure ocky Cre	#12400 ek at S	9700800 C 97	molin-	file:	(413 ft.	bridge) sc97 eptember	1004		
	*** RUN	DATE &	FIME: 10	-04-94	l 07:4	10	:breimer	1994		
XSID:CODE SRD		LEW REW			HF HO	EGL ERR		Q VEL	WSEL	
EXIT :XS -412	*****	118 737		1.22 3.65	**** ****	93.32 *****	85.76 0.54	24400 4.64	92.09	
FULV :FV	413		5277 561070	3.66	0.00	0.02	****** 0.53	4.62	92.90	
<<	<< <the a<="" td=""><td>BOVE RE</td><td>SULTS RE</td><td>FLECT</td><td>"NORMA</td><td>TL" (UNCC</td><td>NSTRICTE</td><td>D) FLOW></td><td>>>>></td><td></td></the>	BOVE RE	SULTS RE	FLECT	"NORMA	TL" (UNCC	NSTRICTE	D) FLOW>	>>>>	
APPR :AS		117 738	5366 569569				****** 0.51		93.80	
								D) FLOW>	>>>>	
	<<< <re< td=""><td>SULTS R</td><td>EFLECTIN</td><td>G THE</td><td>CONSTE</td><td>RICTED FI</td><td>OW FOLLO</td><td>₩>>>></td><td></td><td></td></re<>	SULTS R	EFLECTIN	G THE	CONSTE	RICTED FI	OW FOLLO	₩>>>>		
XSID:CODE SRD	SRDL FLEN	LEW REW		VHD ALPH	HF HO	EGL ERR		Q VEL	WSEL	
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TYPE P	PCD FLOW	C	P/A	LSE	L BI	EN XLA		l.		
XSID:CODE SRD	SRDL FLEN	LEW REW	AREA K		HF HO	EGL ERR		Q VEL	WSEL	-
APPR :AS 444	413 426					94.95 0.00		24400 4.54	93.82	
	M(K) 0.016	K0 561255			_	TEL 3.06				

	PIER SCOUR COMPUTATIONS							
l, J	RO	CKY CREEK AT	SC 97 TN		FOR COUNTY	(A13 FT	RDTDCE!	
		Q100 =	16,900 C	FS	AWC 1	L0-4-1994		
		:=====================================	======	=======	=======	=======	=========	=========
ال ۵۰								
			HYDRAUL	IC VARIABI	LES USED I	N CSU EQU	ATION	
ţ]	PIER NUMBER	-	5	4	3	2	1	
	PIER STATION (FT)				240		360	
	LOCATION OF PIER	LFP			MCR	RFP	RFP	
[_]	Y1: DEPTH (FT)	8.0	9.7		20.0	7.8	13.9	
	V1: VEL. (FPS)	3.7	3.7	5.8	5.8		3.7	
	a: PIER WIDTH (FT)	3.3	3.3	3.3	3.3		3.3	
	L: PIER LENGTH (FT)				22.0	22.0	22.0	
	PIER SHAPE	1	1	1	1	1	1	
[_]	ATTACK ANGLE	8	8	8	8	8	8	
	K1 (SHAPE COEF.)	1.00	1.00	1.00	1.00	1.00	1.00	
ι;	K2 (ANGLE COEF.)	1.45	1.45	1.45	1.45	1.45	1.45	
~	FROUDE NO.	1.45 0.23	0.21	0.23	0.23	0.23	0.17	
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_								
		COMPUTED	SCOUR DE	PTHS USIN	IG CSII EOI	IATTON		
			Joseph Di	11110 0011	10 600 000	ALLON		
~	SCOUR DEPTH (FT)	6 91	7.09	9.47	9.47	6.84	7.40	
[]								
	•							
لـ_ا	MAX SCOUR DEPTH (FT)	7.61	7.80	10.42	10.42	7.53	8.14	
	"MAN GOALD DEDEN!"							

"MAX SCOUR DEPTH" includes an additional 10 percent of the computed CSU scour depth as recommended in HEC 18

CONTRACTION SCOUR COMPUTATIONS

FOR			(
K AT SC 97 IN CHESTER 0 = 16,900 CFS	COUNTY AWC	(413 FT BRIDGE) 10-4-1994	ſ
 =======================================	======	======================================	====

LIVE-BED SCOUR COMPUTATIONS

DISCHARGE (CFS) BOTTOM WIDTH (FT) MANNINGS n AVERAGE DEPTH (FT)	MAIN CHANNEL 14738. 106.0 0.045 19.9	CONTRACTED SECTION 8737. 95.5 0.045
ENERGY SLOPE D50 (FT) FALL VELOCITY (FPS) K1 COEF. K2 COEF.		0.00200 0.0046 0.65 0.64 0.21
COMPUTED DEPTH AT CONTRACTION DEPTH AT MAIN CHANNEL (FT) DEPTH OF CONTRACTION SCOUR		= 13.6 = 19.9 = -6.3

LEFT OVERBANK IN BRIDGE OPENING CLEAR-WATER CONTRACTION SCOUR COMPUTATIONS

DISCHARGE IN CONTRACTED SECTION (CFS)	=	3914.
WIDTH OF CONTRACTED SECTION (FT)	=	152.0
MEDIAN GRAIN SIZE (FT)	=	0.0013
COMPUTED DEPTH OF CONTRACTED SECTION (FT)	=	13.9
AVERAGE FLOOD PLAIN DEPTH (FT)	=	7.8
DEPTH OF CONTRACTION SCOUR (FT)	=	6.0

RIGHT OVERBANK IN BRIDGE OPENING CLEAR-WATER CONTRACTION SCOUR COMPUTATIONS

DISCHARGE IN CONTRACTED SECTION (CFS)	=	4249.
WIDTH OF CONTRACTED SECTION (FT)	=	152.0
MEDIAN GRAIN SIZE (FT)	=	0.0013
COMPUTED DEPTH OF CONTRACTED SECTION (FT)	=	14.9
AVERAGE FLOOD PLAIN DEPTH (FT)	=	7.8
DEPTH OF CONTRACTION SCOUR (FT)	=	7.0

ABUTMENT SCOUR COMPUTATIONS

FOR

ROCKY CREEK AT SC 97 IN CHESTER COUNTY (413 FT BRIDGE) Q100 = 16,900 CFS AWC 10-4-1994

LEFT ABUTMENT SCOUR COMPUTATIONS

ABUTMENT TYPE	1	-VERTICAL WALL
DISCHARGE BLOCKED BY ABUTMENT (CFS)		60.
AREA BLOCKED BY ABUTMENT (SQ FT)		50.0
DEPTH OF FLOW AT ABUTMENT (FT)		3.9
LENGTH OF ABUT. 90 DEG. TO FLOW (FT)		11.0
ABUTMENT SKEW (DEG)		-8
AJUSTED ABUTMENT LENGTH (FT)		13.0
AVERAGE F/P VELOCITY U/S OF ABUT. (FPS)		1.2
FROUDE NUMBER		0.108
K1 COEF.		1.0
K2 COEF.		1.0

DESIGN DEPTH OF SCOUR (FROELICH EQUATION, 1989) (FT) = 7.6

RIGHT ABUTMENT SCOUR COMPUTATIONS

ABUTMENT TYPE	1	-VERTICAL WALL
DISCHARGE BLOCKED BY ABUTMENT (CFS)		442.
AREA BLOCKED BY ABUTMENT (SQ FT)		444.0
DEPTH OF FLOW AT ABUTMENT (FT)		3.7
LENGTH OF ABUT. 90 DEG. TO FLOW (FT)		124.0
ABUTMENT SKEW (DEG)		8
AJUSTED ABUTMENT LENGTH (FT)		121.3
AVERAGE F/P VELOCITY U/S OF ABUT. (FPS)		1.0
FROUDE NUMBER		0.092
K1 COEF.		1.0
K2 COEF.		1.0

DESIGN DEPTH OF SCOUR (FROELICH EQUATION, 1989) (FT) = 12.5

PIER SCOUR COMPUTATIONS

FOR

ROCKY CREEK AT SC 97 IN CHESTER COUNTY (413 FT BRIDGE) $Q500 = 24,400 \text{ CFS} \qquad AWC \qquad 10-4-1994$

=======================================	= 000Q :========	24,400 C	r5	AWC]	 	=========	:== == ==
		HYDRAUL	IC VARIABI	LES USED I	IN CSU EQU	ATION	{
PIER NUMBER PIER STATION (FT) LOCATION OF PIER Y1: DEPTH (FT) V1: VEL. (FPS) a: PIER WIDTH (FT) L: PIER LENGTH (FT) PIER SHAPE ATTACK ANGLE K1 (SHAPE COEF.) K2 (ANGLE COEF.) FROUDE NO.	53 LFP 11.1 4.9 3.3 22.0 1 8	LFP 12.7 4.9 3.3 22.0 1 8 1.00 1.45	MCL 23.0 7.1 3.3 22.0 1 8 1.00 1.45	MCR 23.0 7.1 3.3 22.0 1 8 1.00 1.45	RFP 10.8 5.0 3.3 22.0 1 8 1.00 1.45	RFP 16.9 5.0 3.3 22.0 1 8 1.00 1.45	
	COMPUTED	SCOUR DE	EPTHS USIN	IG CSU EQI	MOITAL		[
SCOUR DEPTH (FT)	8.11	8.25	10.48	10.48	8.17	8.68	[
MAX SCOUR DEPTH (FT)	8.92	9.08	11.53	11.53	8.98	9.54	

"MAX SCOUR DEPTH" includes an additional 10 percent of the computed CSU scour depth as recommended in HEC 18

CONTRACTION SCOUR COMPUTATIONS

FOR

ROCKY CREEK AT SC 97 IN CHESTER COUNTY (413 FT BRIDGE) $Q500 = 24,400 \text{ CFS} \qquad AWC \qquad 10-4-1994$

LIVE-BED SCOUR COMPUTATIONS

DISCHARGE (CFS) BOTTOM WIDTH (FT) MANNINGS n AVERAGE DEPTH (FT)	MAIN CHANNEL 19486. 106.0 0.045 22.9	CONTRACTED SECTION 11033. 95.5 0.045
ENERGY SLOPE		0.00200

D50 (FT) 0.0046
FALL VELOCITY (FPS) 0.65
K1 COEF. 0.64
K2 COEF. 0.21

COMPUTED DEPTH AT CONTRACTED SECTION (FT) = 15.1 DEPTH AT MAIN CHANNEL (FT) = 22.9 DEPTH OF CONTRACTION SCOUR (FT) = -7.9

LEFT OVERBANK IN BRIDGE OPENING CLEAR-WATER CONTRACTION SCOUR COMPUTATIONS

DISCHARGE IN CONTRACTED SECTION (CFS)	=	6541.
WIDTH OF CONTRACTED SECTION (FT)	=	152.0
MEDIAN GRAIN SIZE (FT)	=	0.0013
COMPUTED DEPTH OF CONTRACTED SECTION (FT)	=	21.5
AVERAGE FLOOD PLAIN DEPTH (FT)	=	10.8
DEPTH OF CONTRACTION SCOUR (FT)	=	10.7

RIGHT OVERBANK IN BRIDGE OPENING CLEAR-WATER CONTRACTION SCOUR COMPUTATIONS

WIDTH OF CONTRACTED SECTION (FT)	= =	6826. 152.0 0.0013
DEDELL OF COMMENCE COMMENCE (TO)	=	22.3 10.8 11.5

ABUTMENT SCOUR COMPUTATIONS

FOR

ROCKY CREEK AT SC 97 IN CHESTER COUNTY

Q500 = 24,400 CFS

(413 FT BRIDGE)

AWC 10-4-1994 _______

LEFT	ABUTMENT
SCOUR CO	ЭМОТФАФІІЧМО

ABUTMENT TYPE DISCHARGE BLOCKED BY ABUTMENT (CFS) AREA BLOCKED BY ABUTMENT (SQ FT) DEPTH OF FLOW AT ABUTMENT (FT) LENGTH OF ABUT. 90 DEG. TO FLOW (FT) ABUTMENT SKEW (DEG)	6.9
AJUSTED ABUTMENT LENGTH (FT) AVERAGE F/P VELOCITY U/S OF ABUT. (FPS) FROUDE NUMBER K1 COEF. K2 COEF.	43.5 1.3 0.090 1.0
DESIGN DEPTH OF SCOUR (FROELICH EQUATION	ON, 1989) (FT) = 14.7
7.7	GHT ABUTMENT JR COMPUTATIONS

ABUTMENT TYPE	1	-VERTICAL WALL
DISCHARGE BLOCKED BY ABUTMENT (CFS)		955.
AREA BLOCKED BY ABUTMENT (SQ FT)		781.0
DEPTH OF FLOW AT ABUTMENT (FT)		6.7
LENGTH OF ABUT. 90 DEG. TO FLOW (FT)		158.0
ABUTMENT SKEW (DEG)		8
AJUSTED ABUTMENT LENGTH (FT)		116.7
AVERAGE F/P VELOCITY U/S OF ABUT. (FPS)		1.2
FROUDE NUMBER		0.083
K1 COEF.		1.0
K2 COEF.		1.0

DESIGN DEPTH OF SCOUR (FROELICH EQUATION, 1989) (FT) = 18.2

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United States Department of the Interior



GEOLOGICAL SURVEY

Water Resources Division Stephenson Center, Suite 129 720 Gracern Road Columbia, SC 29210-7651

October 12, 1994

William H. Hulbert, P.E. Hydraulic Engineer South Carolina Department of Transportation 955 Park Street Columbia, South Carolina 29202

Dear Mr. Hulbert:

We are pleased to transmit to you another report of the Level II Bridge Scour Program titled, "Level II bridge scour analysis for structure 124009700800 on Route SC 97, crossing Rocky Creek in Chester County, South Carolina," by Andy W. Caldwell and Michael G. Zalants. The technical aspects have been reviewed by the South Carolina District Surface-Water Specialist and the report has been approved by the South Carolina Bridge Scour Project Chief.

If you have any questions concerning this report please contact me (750-6101) or Michael G. Zalants (750-6159) and we will be glad to assist you.

Sincerely,

Andy W. Caldwell

Civil Engineer

Enclosure

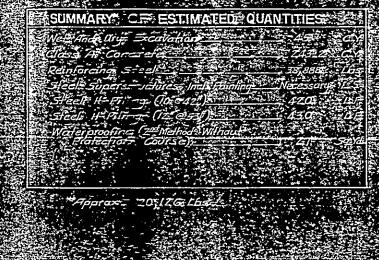


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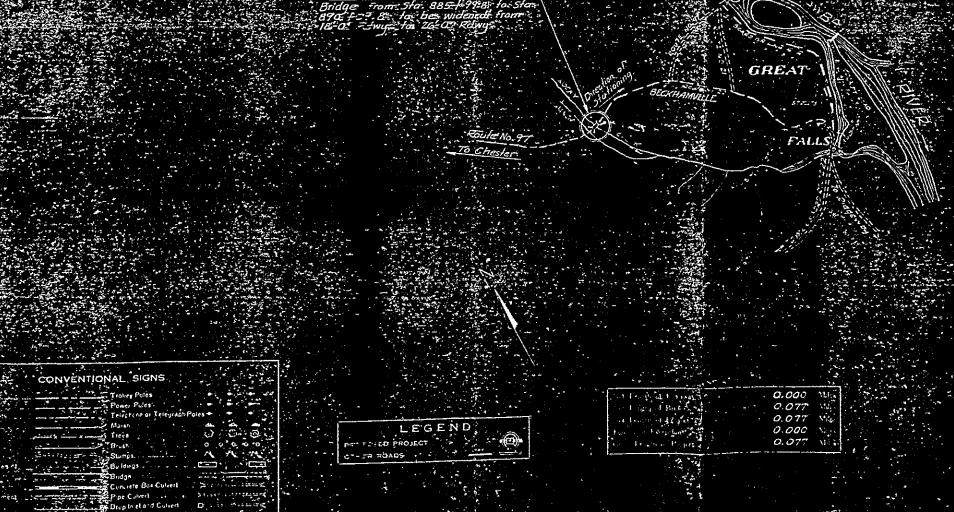
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FEDERAL AID PROJECT NO. F-3. SC DOCKET NO. 12:328 ROUTE NO. 97

WIDENING BRIDGE OVER ROCKY CREE







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SUMMARY: OF ESTIMATED: QUANTILES	
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Total 45 Z43.Z 38,888 120,126 4ZA 430	
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PILE RECORD ON DOCKET NO. 22.322

NT I	FOOTING	PILE:	DATE	DIAMETER. BUTT		PENETRA-			FIFWITTEN	EI EVATION	LENGTH	- noicina	4 EMICTU	0.0	C.O. AT 0.40 OF UNIT PRICE	7.72	PF Design	Pro Alemania de	PILE	DATE ;	DIAMETER	DIAMETER	PENETRA-	PEN. PER	BEARING		ELEVATION	
5	- <u>-</u> - 1 - 1	18	8-10-54	Stool N. P.	ling 12 05	19.95	0250Y	3960	213.15	245.529	32.38	40.00	7.62	3238	SPECIE SE	OF UNIT FIRE	ALC:			DRIVEN 또 (전다	BUTT	TIP TO	TION	BLOW :	DEARING.		CUT-OFF	
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