

Sally Lott
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U.S. DEPARTMENT OF THE INTERIOR

U.S. GEOLOGICAL SURVEY

LEVEL II BRIDGE SCOUR ANALYSIS FOR STRUCTURE 124022300100
ON ROUTE SC 223, CROSSING FISHING CREEK IN CHESTER
COUNTY, SOUTH CAROLINA

By Michael G. Zalants and Andy W. Caldwell

9-4

Prepared in cooperation with the
SOUTH CAROLINA DEPARTMENT
OF TRANSPORTATION



Columbia, South Carolina

1994

UNIT ABBREVIATIONS

cubic foot per second	ft ³ /s
feet per second	ft/s
foot	ft
mile	mi
millimeter	mm
square foot	ft ²
square mile	mi ²

OTHER ABBREVIATIONS

downstream	D/S
upstream	U/S
flood plain	f/p
median diameter of bed material	D ₅₀
Water -Surface Profile computation model	WSPRO
South Carolina Department of Transportation	SCDOT

In this report, the words "right" and "left" refer to directions that would be reported by an observer facing downstream.

Sea level: In this report, "sea level" refers to the National Geodetic Vertical Datum of 1929-- a geodetic datum derived from a general adjustment of the first-order level nets of the United States and Canada, formerly called Sea Level Datum of 1929.

Pile penetration depths were obtained from SCDOT bridge plans. Scour depth calculations indicate that pile tip exposure will occur at bent 3 during the 100-year discharge and at bents 3, 4, 7, 9, and 10 during the 500-year discharge. Scour caused by the 100- and 500-year discharges will undermine bent 3 by 4.6 and 14.4 ft, respectively. Scour caused by the 500-year discharge will undermine bents 4, 7, 9, and 10 by 8.8, 2.2, 0.4, and 0.8 ft, respectively.

It should be noted that the SCDOT bridge plan borings (docket number 12.250) show subsurface rock that could affect the scour depths shown in this study. For more information, see the SCDOT bridge plans in the pocket at the back of the report.

Table 2. --Remaining pile/footing penetration at piers/bents for the 500-year discharge at structure 124022300100 on Route SC 223, crossing Fishing Creek in Chester County, South Carolina

Pier/bent number	Station from ² left end of bridge (feet)	Pile tip/ ³ footing elevation, SCDOT datum (feet)	Pile tip/ footing elevation, USGS datum (feet)	Ground elevation at pier/bent, USGS datum (feet)	Total ⁴ scour depth (feet)	Elevation of scour, USGS datum (feet)	Remaining ⁵ pile/footing penetration (feet)
500-year discharge is 34,200 cubic feet per second							
11	25	389.9	50.9	86.0	2.5	83.5	32.6
10	50	388.6	49.6	75.2	26.4	48.8	-0.8
9	75	388.4	49.4	75.4	26.4	49.0	-0.4
8	100	388.3	49.3	75.9	26.4	49.5	0.2
7	125	386.5	47.5	76.0	30.7	45.3	-2.2
6	190	385.8	46.8	66.7	11.0	55.7	8.9
5	270	386.5	47.5	59.3	10.2	49.1	1.6
4	335	395.6	56.6	77.2	29.4	47.8	-8.8
3	360	409.4	70.4	80.0	24.0	56.0	-14.4
2	385	422.2	83.2	92.8	0	92.8	9.6

¹ Pier/bent number corresponds to South Carolina Department of Transportation (SCDOT) bridge plans.

² Stations are determined from left to right looking downstream.

³ Pile tip/footing elevations obtained from SCDOT bridge plans. The maximum elevation at each pier/bent is used.

⁴ Total scour depth is the sum of the contraction and pier/bent scour depths.

⁵ A negative number signifies undermining of pile tip/footing.

NOTE: The SCDOT bridge plan borings (docket number 12.250) show subsurface rock that could reduce the scour depths shown in the above table. For more information, see the SCDOT plans in report pocket.

Table 4. --Cumulative scour depths at piers/bents for the 500-year discharge at structure 124022300100, on Route SC 223, crossing Fishing Creek in Chester County, South Carolina

Pier/bent ¹ number	Station from ² left end of bridge (feet)	Contraction scour depth (feet)	Pier/bent scour depth without debris (feet)	Total ³ scour depth without debris (feet)
500-year discharge is 34,200 cubic feet per second				
11	25	0.0	2.5	2.5
10	50	23.6	2.8	26.4
9	75	23.6	2.8	26.4
8	100	23.6	2.8	26.4
7	125	23.6	7.1	30.7
6	190	0.0 ⁴	11.0	11.0
5	270	0.0 ⁴	10.2	10.2
4	335	21.3	8.1	29.4
3	360	21.3	2.7	24.0
2	385	0.0	0.0	0.0

¹ Pier/bent number corresponds to South Carolina Department of Transportation (SCDOT) bridge plans.

² Stations are determined from left to right looking downstream.

³ Total scour depth is the sum of the contraction and pier/bent scour depths.

⁴ The calculated contraction scour is a negative value, but was set equal to zero to reflect a more reasonable estimate of scour during peak flood conditions.

NOTE: The SCDOT bridge plan borings (docket number 12.250) show subsurface rock that could reduce the scour depths shown in the above table. For more information, see the SCDOT plans in report pocket.

NOTE: The pier and contraction scour equations used in this scour analysis were those recommended in Hydraulic Engineering Circular 18 (Richardson and others, 1993). Scour depths were calculated assuming an infinite depth of erosive material and a homogeneous particle-size distribution.

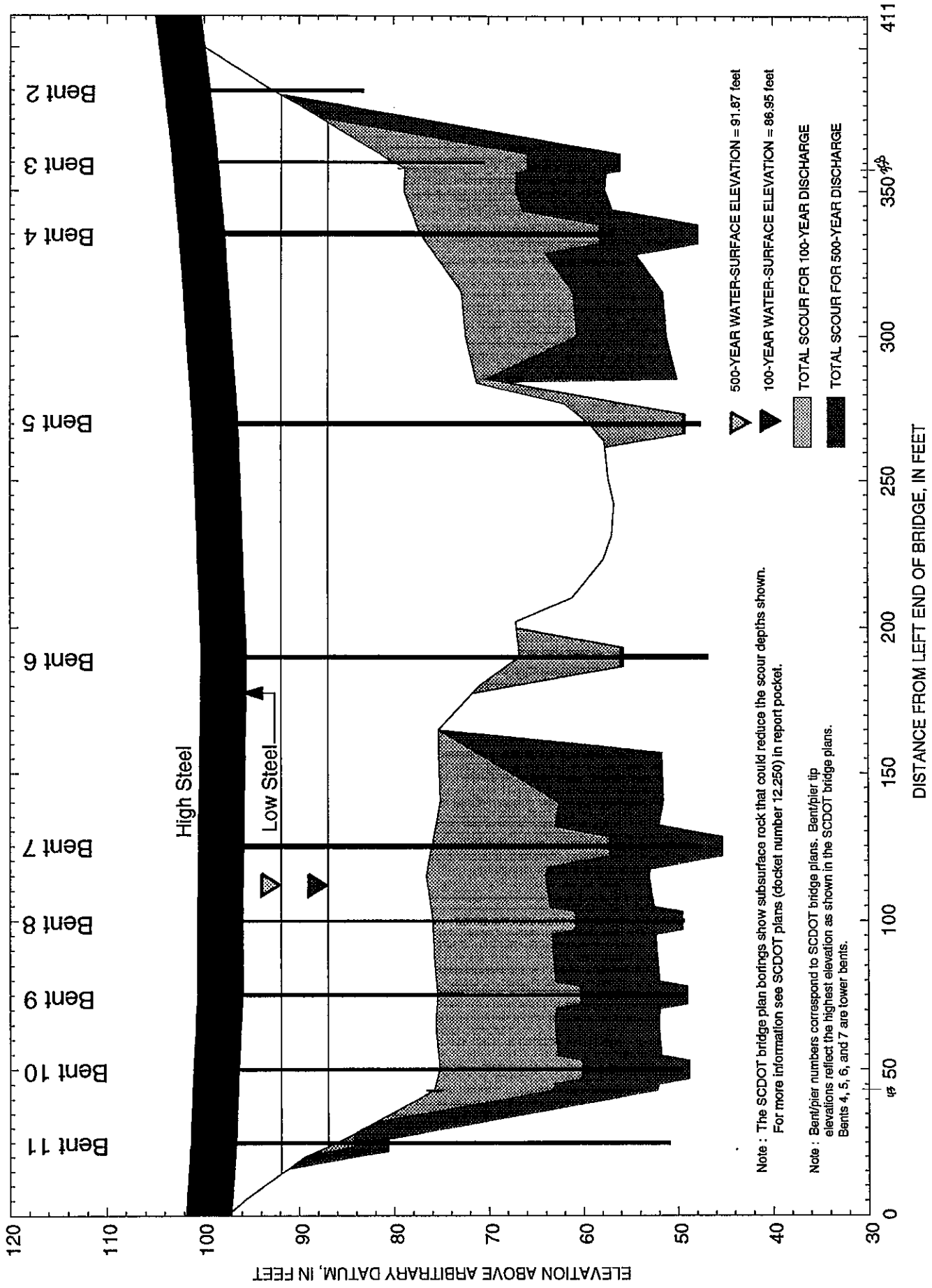


Figure 2.--Total scour depths for the 100-year and 500-year discharges at the U/S face of structure 124022300100 on Route SC 223, crossing Fishing Creek in Chester County, South Carolina.

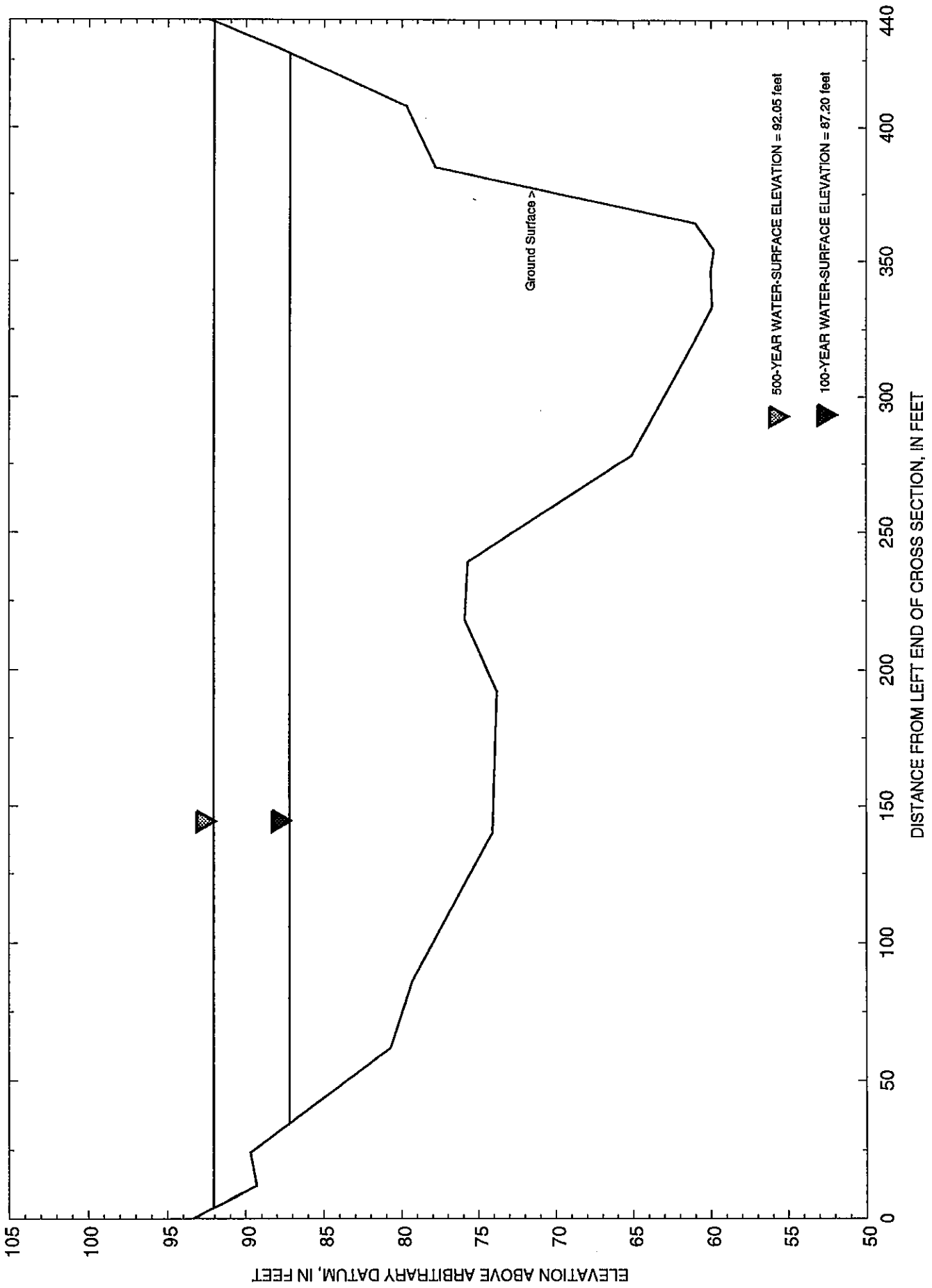


Figure 3.--Approach cross section of structure 124022300100 on Route SC 223, crossing Fishing Creek in Chester County, South Carolina.



Figure 4.--Structure 124022300100 on Route SC 223, crossing Fishing Creek in Chester County, South Carolina as viewed from the upstream right bank (July 19, 1990).

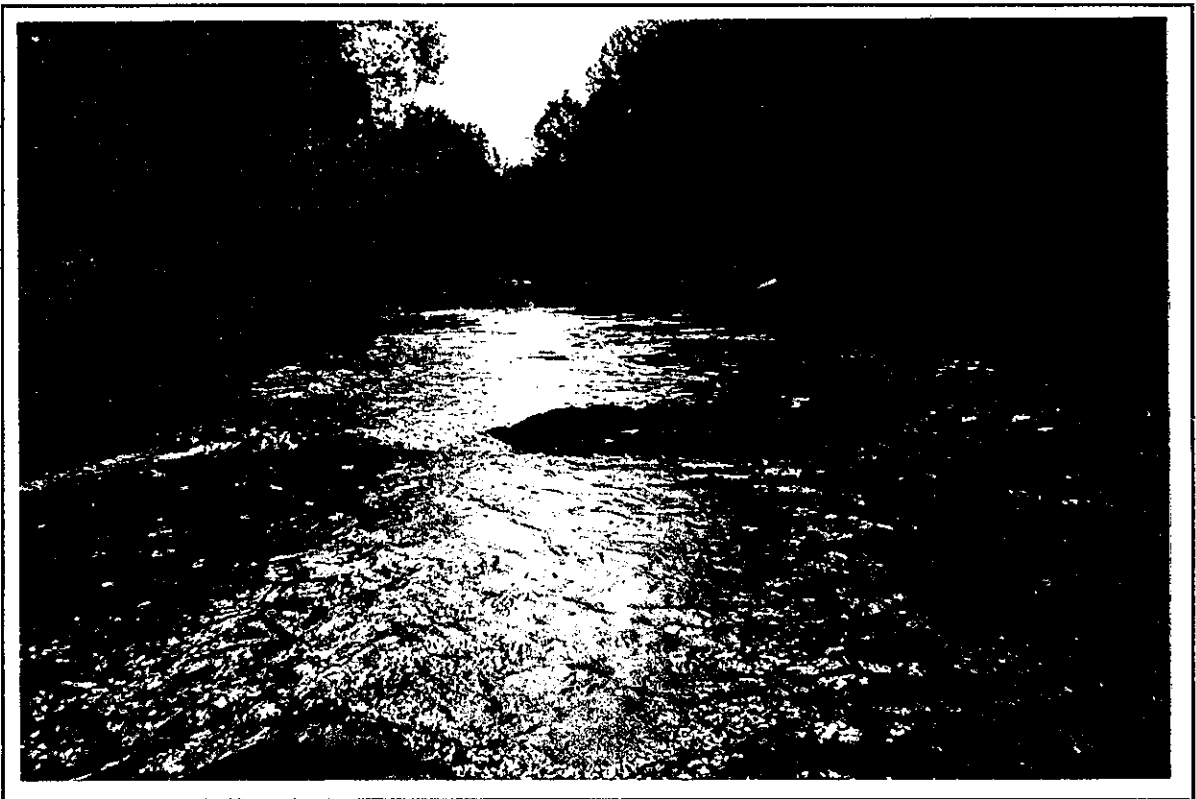


Figure 5.--Downstream channel as viewed from the channel D/S of structure 124022300100 on Route SC 223, crossing Fishing Creek in Chester County, South Carolina (May 18, 1993).



Figure 6.--Upstream channel as viewed from structure 124022300100 on Route SC 223, crossing Fishing Creek in Chester County, South Carolina (May 18, 1993).

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Description of Flood Plain

General topography Gently rolling Piedmont topography with a right flood plain that rises quickly.

Flood-plain conditions at bridge site: downstream (D/S), upstream (U/S)

Date of inspection 5-18-1993

D/S left: Moderate hardwoods with sparse undergrowth

D/S right: Clear-cut area with tall grass, then moderate hardwoods at right edge

U/S left: Light to moderate hardwoods with moderate undergrowth

U/S right: Moderate hardwoods with moderate undergrowth

Description of Channel

Average top width 135 ft *Average depth* 18.4 ft

Predominant bed material sand *Bank material* silty sand

Stream type (straight, meandering, braided, swampy, channelized) meandering

Vegetative cover on channel banks near bridge: Date of inspection 7-19-1990

D/S left: Full coverage of woody and herbaceous vegetation

D/S right: Light coverage of woody and herbaceous vegetation

U/S left: Full coverage of woody and herbaceous vegetation

U/S right: Light coverage of woody and herbaceous vegetation

Do banks appear stable? Yes * *If not, describe location and type of instability and date of observation.* *Normal fluvial bank erosion was observed on the downstream right bank during the Level I inspection of 7-19-1990.

Describe any obstructions in channel and date of observation. A run-of-the-river dam is located across the channel approximately 550 ft upstream of the Route SC 223 crossing. The dam has no effect on the hydrology at that crossing.

Bridge Hydraulics

Average embankment elevation 100.8 ft

Average low steel elevation 97.0 ft

100-year discharge 23,500 ft³/s

Water-surface elevation at D/S bridge face 86.95 ft

Area of flow at D/S bridge face 5,335 ft²

Average velocity in bridge opening 4.40 ft/s

Maximum WSPRO tube velocity at bridge 6.85 ft/s

Water-surface elevation at Approach section with bridge 87.20 ft

Water-surface elevation at Approach section without bridge 87.15 ft

Amount of backwater caused by bridge 0.05 ft

500-year discharge 34,200 ft³/s

Water-surface elevation at D/S bridge face 91.87 ft

Area of flow at D/S bridge face 7,104 ft²

Average velocity in bridge opening 4.81 ft/s

Maximum WSPRO tube velocity at bridge 7.00 ft/s

Water-surface elevation at Approach section with bridge 92.05 ft

Water-surface elevation at Approach section without bridge 92.03 ft

Amount of backwater caused by bridge 0.02 ft

The live-bed contraction scour equation indicates the deposition of sediment in the channel at the bridge during the 100- and 500-year floods. (See negative scour values determined in scour calculations included at the end of the report). However, it seems unreasonable to expect sediment deposition at the bridge during peak flood conditions. Therefore, the negative scour values were set equal to zero as reflected in tables 1 through 4 and figure 2.

The abutments are not protected by riprap, therefore abutment scour was calculated using the Froehlich (1989) live-bed abutment scour equation. There is no contraction of flow at the right abutment during the 100- or 500-year discharges; therefore, the flood discharges do not produce abutment scour at the right abutment. It should be noted that the SCDOT bridge plans (docket number 12.250) show that the right abutment is the actual ground surface, not fill material, in that area.

It should be noted that the SCDOT bridge plan borings (docket number 12.250) show subsurface rock that could affect the scour depths shown in this study. For more information, see the SCDOT bridge plans in the pocket at the back of the report.

WSPRO INPUT FILE -- Continued

*
 * Survey data for an approach cross section surveyed at 570 ft U/S
 * of D/S bridge face. Distance is determined from survey data.
 *

XT	SURV2	570	0.0008							
GR		0	93.5	12	89.4	24	89.8	62	80.8	86
GR		140	74.2	192	73.9	218	76.0	239	75.8	278
GR		321	61.1	333	60.0	346	60.1	354	59.9	364
GR		385	77.9	408	79.8	418	83.7	430	88.3	440

*
 *

AS APPR 440

GT

*

N 0.15 0.06 0.16

SA 239 385

*

BP 74

PX

*

HP 1 BRIDG	86.95	0	86.95	
HP 2 BRIDG	86.97	0	86.97	23500
HP 1 APPR	87.20	0	87.20	
HP 2 APPR	87.20	0	87.20	23500
HP 1 BRIDG	91.87	0	91.87	
HP 2 BRIDG	91.89	0	91.89	34200
HP 1 APPR	92.05	0	92.05	
HP 2 APPR	92.05	0	92.05	34200

EX

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WSPRO OUTPUT --Continued

WSPRO FEDERAL HIGHWAY ADMINISTRATION - U. S. GEOLOGICAL SURVEY
 V060188 MODEL FOR WATER-SURFACE PROFILE COMPUTATIONS

Structure No. 124022300100 (411 ft. Bridge)
 Fishing Creek at SC 223 File: fish.sc223
 Chester County, South Carolina MGZ 8/94

*** RUN DATE & TIME: 08-23-94 10:43

CROSS-SECTION PROPERTIES: ISEQ = 4; SECID = APPR ; SRD = 440.

WSEL	SA#	AREA	K	TOPW	WETP	ALPH	LEW	REW	QCR
	1	2079.	96594.	204.	206.				37627.
	2	3230.	610925.	146.	154.				86197.
	3	267.	8283.	42.	44.				3800.
87.20		5576.	715802.	393.	403.	1.87	35.	427.	87135.

WSPRO FEDERAL HIGHWAY ADMINISTRATION - U. S. GEOLOGICAL SURVEY
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VELOCITY DISTRIBUTION: ISEQ = 4; SECID = APPR ; SRD = 440.

	WSEL	LEW	REW	AREA	K	Q	VEL
	87.20	34.5	427.4	5575.9	715802.	23500.	4.21
X STA.	34.5		149.3	197.5	247.4	263.5	274.2
A(I)		951.2	639.6	595.0	256.4	210.3	
V(I)		1.24	1.84	1.97	4.58	5.59	
X STA.	274.2		282.7	290.4	297.8	305.0	311.6
A(I)		186.6	176.3	176.7	173.0	166.9	
V(I)		6.30	6.67	6.65	6.79	7.04	
X STA.	311.6		318.2	324.6	330.9	336.8	342.9
A(I)		168.3	167.0	169.4	160.6	166.3	
V(I)		6.98	7.04	6.94	7.32	7.07	
X STA.	342.9		348.8	354.7	360.9	368.2	427.4
A(I)		162.2	161.1	165.9	184.6	538.7	
V(I)		7.24	7.30	7.08	6.37	2.18	

WSPRO OUTPUT --Continued

WSPRO FEDERAL HIGHWAY ADMINISTRATION - U. S. GEOLOGICAL SURVEY
 V060188 MODEL FOR WATER-SURFACE PROFILE COMPUTATIONS

Structure No. 124022300100 (411 ft. Bridge)
 Fishing Creek at SC 223 File: fish.sc223
 Chester County, South Carolina MGZ 8/94

*** RUN DATE & TIME: 08-23-94 10:43

CROSS-SECTION PROPERTIES: ISEQ = 4; SECID = APPR ; SRD = 440.

WSEL	SA#	AREA	K	TOPW	WETP	ALPH	LEW	REW	QCR
	1	3151.	175653.	235.	237.				65455.
	2	3938.	850109.	146.	154.				116046.
	3	502.	19998.	54.	57.				8660.
92.05		7590.	1045760.	435.	447.	2.02	4.	439.	126395.

WSPRO FEDERAL HIGHWAY ADMINISTRATION - U. S. GEOLOGICAL SURVEY
 V060188 MODEL FOR WATER-SURFACE PROFILE COMPUTATIONS

Structure No. 124022300100 (411 ft. Bridge)
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 Chester County, South Carolina MGZ 8/94

*** RUN DATE & TIME: 08-23-94 10:43

VELOCITY DISTRIBUTION: ISEQ = 4; SECID = APPR ; SRD = 440.

	WSEL	LEW	REW	AREA	K	Q	VEL
	92.05	3.9	439.2	7590.0	1045760.	34200.	4.51
X STA.	3.9		133.2	176.8		220.7	251.6
A(I)		1301.1		784.3		768.2	524.8
V(I)		1.31		2.18		2.23	3.26
X STA.	265.2		275.4	284.0		292.3	300.2
A(I)		255.4		230.9		231.7	226.4
V(I)		6.70		7.40		7.38	7.55
X STA.	307.6		314.8	322.1		328.9	335.5
A(I)		218.1		223.1		214.7	213.5
V(I)		7.84		7.66		7.96	8.01
X STA.	342.2		348.8	355.3		362.1	371.3
A(I)		211.6		210.0		216.0	264.9
V(I)		8.08		8.14		7.92	6.46

WSPRO OUTPUT --Continued

WSPRO FEDERAL HIGHWAY ADMINISTRATION - U. S. GEOLOGICAL SURVEY
 V060188 MODEL FOR WATER-SURFACE PROFILE COMPUTATIONS

Structure No. 124022300100 (411 ft. Bridge)
 Fishing Creek at SC 223 File: fish.sc223
 Chester County, South Carolina MGZ 8/94
 *** RUN DATE & TIME: 08-23-94 10:43

XSID:CODE	SRDL	LEW	AREA	VHD	HF	EGL	CRWS	Q	WSEL
SRD	FLEN	REW	K	ALPH	HO	ERR	FR#	VEL	
EXIT :XS	*****	-6.	9211.	0.46	*****	91.85	75.80	34200.	91.39
-411.	*****	604.	1216749.	2.16	*****	*****	0.25	3.71	
FULV :FV	411.	-6.	9211.	0.46	0.32	92.18	*****	34200.	91.72
0.	411.	604.	1216707.	2.16	0.00	0.00	0.25	3.71	
<<<<<THE ABOVE RESULTS REFLECT "NORMAL" (UNCONSTRICTED) FLOW>>>>>									
APPR :AS	440.	4.	7583.	0.64	0.40	92.67	*****	34200.	92.03
440.	440.	439.	1044465.	2.02	0.09	0.00	0.27	4.51	
<<<<<THE ABOVE RESULTS REFLECT "NORMAL" (UNCONSTRICTED) FLOW>>>>>									

<<<<<RESULTS REFLECTING THE CONSTRICTED FLOW FOLLOW>>>>>

XSID:CODE	SRDL	LEW	AREA	VHD	HF	EGL	CRWS	Q	WSEL
SRD	FLEN	REW	K	ALPH	HO	ERR	FR#	VEL	
BRIDG:BR	411.	14.	7104.	0.36	0.38	92.23	77.97	34200.	91.87
0.	411.	383.	1737053.	1.00	0.00	0.01	0.19	4.81	
TYPE	PPCD	FLOW	C	P/A	LSEL	BLEN	XLAB	XRAB	
3.	1.	1.	1.000	0.029	97.00	*****	*****	*****	

XSID:CODE	SRDL	LEW	AREA	VHD	HF	EGL	CRWS	Q	WSEL
SRD	FLEN	REW	K	ALPH	HO	ERR	FR#	VEL	
APPR :AS	411.	4.	7589.	0.64	0.44	92.69	77.54	34200.	92.05
440.	413.	439.	1045625.	2.02	0.02	0.00	0.27	4.51	
M(G)	M(K)	KQ	XLKQ	XRKQ	OTEL				
0.153	0.000	1045314.	71.	440.	91.61				

<<<<<END OF BRIDGE COMPUTATIONS>>>>>

CONTRACTION SCOUR COMPUTATIONS
FOR

Fishing Creek at SC 223; Chester County; Struc.# 124022300100; MGZ 8-24-94
Scour Analysis for Q100

=====

LEFT OVERBANK IN BRIDGE OPENING
CLEAR-WATER CONTRACTION SCOUR COMPUTATIONS

DISCHARGE IN CONTRACTED SECTION (CFS) = 4792.
WIDTH OF CONTRACTED SECTION (FT) = 122.0
MEDIAN GRAIN SIZE (FT) = 0.0008

COMPUTED DEPTH OF CONTRACTED SECTION (FT) = 22.8
AVERAGE FLOOD PLAIN DEPTH (FT) = 10.2
DEPTH OF CONTRACTION SCOUR (FT) = 12.6

RIGHT OVERBANK IN BRIDGE OPENING
CLEAR-WATER CONTRACTION SCOUR COMPUTATIONS

DISCHARGE IN CONTRACTED SECTION (CFS) = 3090.
WIDTH OF CONTRACTED SECTION (FT) = 74.0
MEDIAN GRAIN SIZE (FT) = 0.0008

COMPUTED DEPTH OF CONTRACTED SECTION (FT) = 24.0
AVERAGE FLOOD PLAIN DEPTH (FT) = 12.2
DEPTH OF CONTRACTION SCOUR (FT) = 11.8

LIVE-BED SCOUR COMPUTATIONS

	MAIN CHANNEL	CONTRACTED SECTION
DISCHARGE (CFS)	20057.	15618.
BOTTOM WIDTH (FT)	146.0	115.8
MANNINGS n	0.045	0.045
AVERAGE DEPTH (FT)	26.8	

ENERGY SLOPE	0.00110
D50 (FT)	0.0010
FALL VELOCITY (FPS)	0.14
K1 COEF.	0.69
K2 COEF.	0.37

COMPUTED DEPTH AT CONTRACTED SECTION (FT) = 25.4
DEPTH AT MAIN CHANNEL (FT) = 26.8
DEPTH OF CONTRACTION SCOUR (FT) = -1.4

PIER SCOUR COMPUTATIONS
 FOR
 Fishing Creek at SC 223; Chester County; Struc. #124022300100; MGZ 8-24-94
 Scour Analysis for Q500

=====

HYDRAULIC VARIABLES USED IN CSU EQUATION

PIER NUMBER	11	10	9	8	7	6	5	4
PIER STATION (FT)	25	50	75	100	125	190	270	335
LOCATION OF PIER	lfp	lfp	lfp	lfp	lfp	mcl	mcr	rfp
Y1: DEPTH (FT)	5.9	16.7	16.5	16.0	15.9	35.2	35.2	14.7
V1: VEL. (FPS)	4.2	4.2	4.2	4.2	4.2	6.3	6.3	4.8
a: PIER WIDTH (FT)	1.0	1.0	1.0	1.0	3.6	4.6	4.1	4.1
L: PIER LENGTH (FT)	5.0	5.0	5.0	5.0	3.6	4.6	4.1	4.1
PIER SHAPE	5	5	5	5	1	1	1	1
ATTACK ANGLE	0	0	0	0	0	0	0	0
K1 (SHAPE COEF.)	1.00	1.00	1.00	1.00	1.10	1.10	1.10	1.10
K2 (ANGLE COEF.)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FROUDE NO.	0.31	0.18	0.18	0.19	0.19	0.19	0.19	0.22

COMPUTED SCOUR DEPTHS USING CSU EQUATION

SCOUR DEPTH (FT)	2.24	2.58	2.58	2.57	6.48	10.03	9.31	7.34
MAX SCOUR DEPTH (FT)	2.47	2.84	2.83	2.82	7.13	11.04	10.24	8.07

HYDRAULIC VARIABLES USED IN CSU EQUATION

PIER NUMBER	3	2
PIER STATION (FT)	360	385
LOCATION OF PIER	rfp	rfp
Y1: DEPTH (FT)	11.9	0.0
V1: VEL. (FPS)	4.8	0.0
a: PIER WIDTH (FT)	0.8	1.0
L: PIER LENGTH (FT)	4.0	5.0
PIER SHAPE	1	5
ATTACK ANGLE	0	0
K1 (SHAPE COEF.)	1.10	1.00
K2 (ANGLE COEF.)	1.00	1.00
FROUDE NO.	0.24	0.00

COMPUTED SCOUR DEPTHS USING CSU EQUATION

SCOUR DEPTH (FT)	2.46	0.00
MAX SCOUR DEPTH (FT)	2.71	0.00

"MAX SCOUR DEPTH" includes an additional 10 percent of the computed CSU scour depth as recommended in HEC 18

ABUTMENT SCOUR COMPUTATIONS

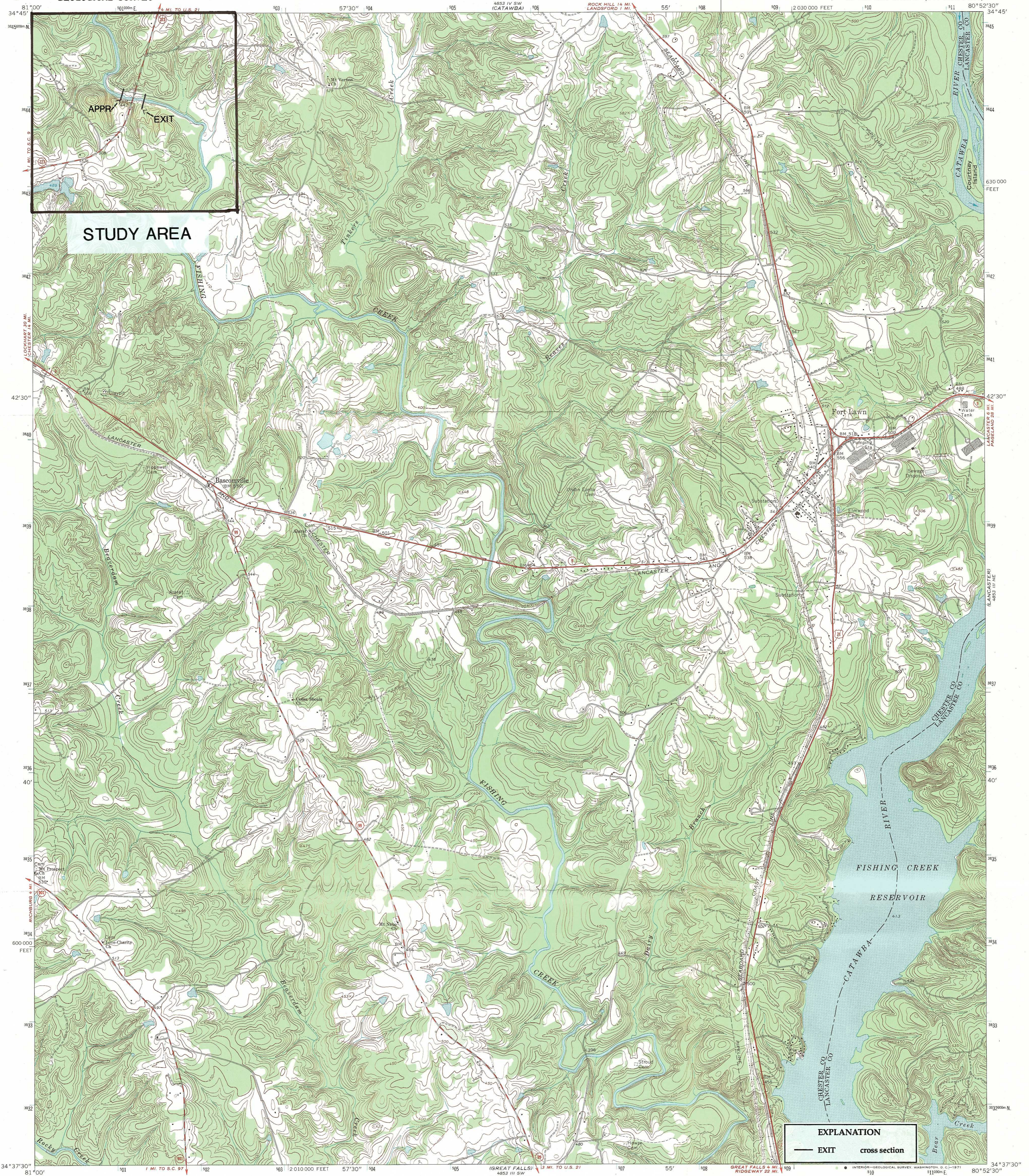
FOR

Fishing Creek at SC 223; Chester County; Struc. #124022300100; MGZ 8-24-94
Scour Analysis for Q500

=====

LEFT ABUTMENT
SCOUR COMPUTATIONS

ABUTMENT TYPE	3 -SPILL THROUGH
DISCHARGE BLOCKED BY ABUTMENT (CFS)	1496.
AREA BLOCKED BY ABUTMENT (SQ FT)	1138.0
DEPTH OF FLOW AT ABUTMENT (FT)	16.1
LENGTH OF ABUT. 90 DEG. TO FLOW (FT)	113.0
ABUTMENT SKEW (DEG)	0
AJUSTED ABUTMENT LENGTH (FT)	70.7
AVERAGE F/P VELOCITY U/S OF ABUT. (FPS)	1.3
FROUDE NUMBER	0.058
K1 COEF.	0.6
K2 COEF.	1.0
DESIGN DEPTH OF SCOUR (FROELICH EQUATION, 1989) (FT)	= 22.8



STUDY AREA

EXPLANATION
— EXIT cross section

ROAD CLASSIFICATION
Primary highway, all weather, hard surface
Secondary highway, all weather, hard surface
Light-duty road, all weather, improved surface
Unimproved road, fair or dry weather

U. S. Route
State Route

Mapped, edited, and published by the Geological Survey
Control by USGS, USC&GS, and South Carolina Geodetic Survey
Topography by photogrammetric methods from aerial photographs
photographs taken 1964. Field checked 1969.
Polyconic projection. 1927 North American datum
10,000-foot grid based on South Carolina coordinate system, north zone
1000-meter Universal Transverse Mercator grid ticks,
zone 17, shown in blue
Fine red dashed lines indicate selected fence and field lines where generally
visible on aerial photographs. This information is unchecked

UTM GRID AND 1969 MAGNETIC NORTH
DECLINATION AT CENTER OF SHEET

SCALE 1:24,000
CONTOUR INTERVAL 10 FEET
DATUM IS MEAN SEA LEVEL

QUADRANGLE LOCATION

Figure 1.—Topography of study area and location of cross sections used in WSPRO analysis for structure 124022300100 on Route SC 223, crossing Fishing Creek in Chester County, South Carolina.

FORT LAWN, S. C.
N3437.5—W8052.5/7.5
1969
AMS 4853 III NW—SERIES V846