

SCOUR EVALUATION
FOR
SOUTH CAROLINA DEPARTMENT
OF TRANSPORTATION
ON
STRUCTURE #127020100100
S-201 OVER LITTLE SANDY RIVER
CHESTER COUNTY, SC

Prepared by:

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February 1995

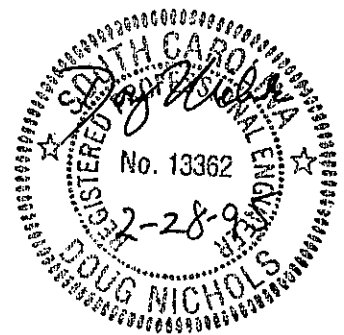


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SCOUR EVALUATION FOR S-201 OVER LITTLE SANDY RIVER

1.0 STREAM STABILITY

1.1 Stream Characteristics

A field investigation recorded the stream characteristics on Figure 1 and on forms contained in Appendix 1. The investigation found that the overbank areas are well grassed or forested and the channel area is free from vegetation. The stream is described as a small perennial but flashy stream. A portion of the USGS Quad map is presented as Figure 2 for location purposes.

1.2 Land Use Changes

The bridge site is approximately 8.4 miles southwest of Chester, South Carolina. The land use in the drainage basin is rural. No future land use changes are expected for the life of the structure.

1.3 Overall Stream Stability

The overall stream stability appears to be stable. The 2-Year discharge and the stream slope have been plotted in the transitional zone on Figure 3 (Figure 8 in HEC-20⁽¹⁾). This shows that a sand bed stream at this site would be in transition between a braided and a meandering stream. The actual stream conditions include moderate vegetation which will influence the behavior of the stream.

1.4 Lateral Stability

The stream alignment is perpendicular to the bridge opening. Photographs of the site (Appendix 2) show that the stream banks are vegetated and relatively stable during average flow conditions.

1.5 Vertical Stability

The stream bed profile is relatively stable. This is evidenced by similar channel depths for the 1978 construction plans and the channel depths measured for this report.

1.6 Channel Response to Change

The channel may still be responding to the change resulting from the construction of the bridge. No site changes are anticipated at this time which would further influence the stream behavior.



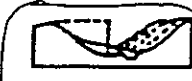



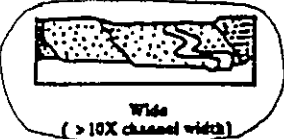
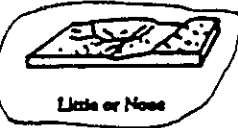










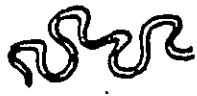




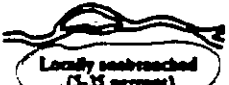

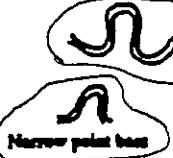



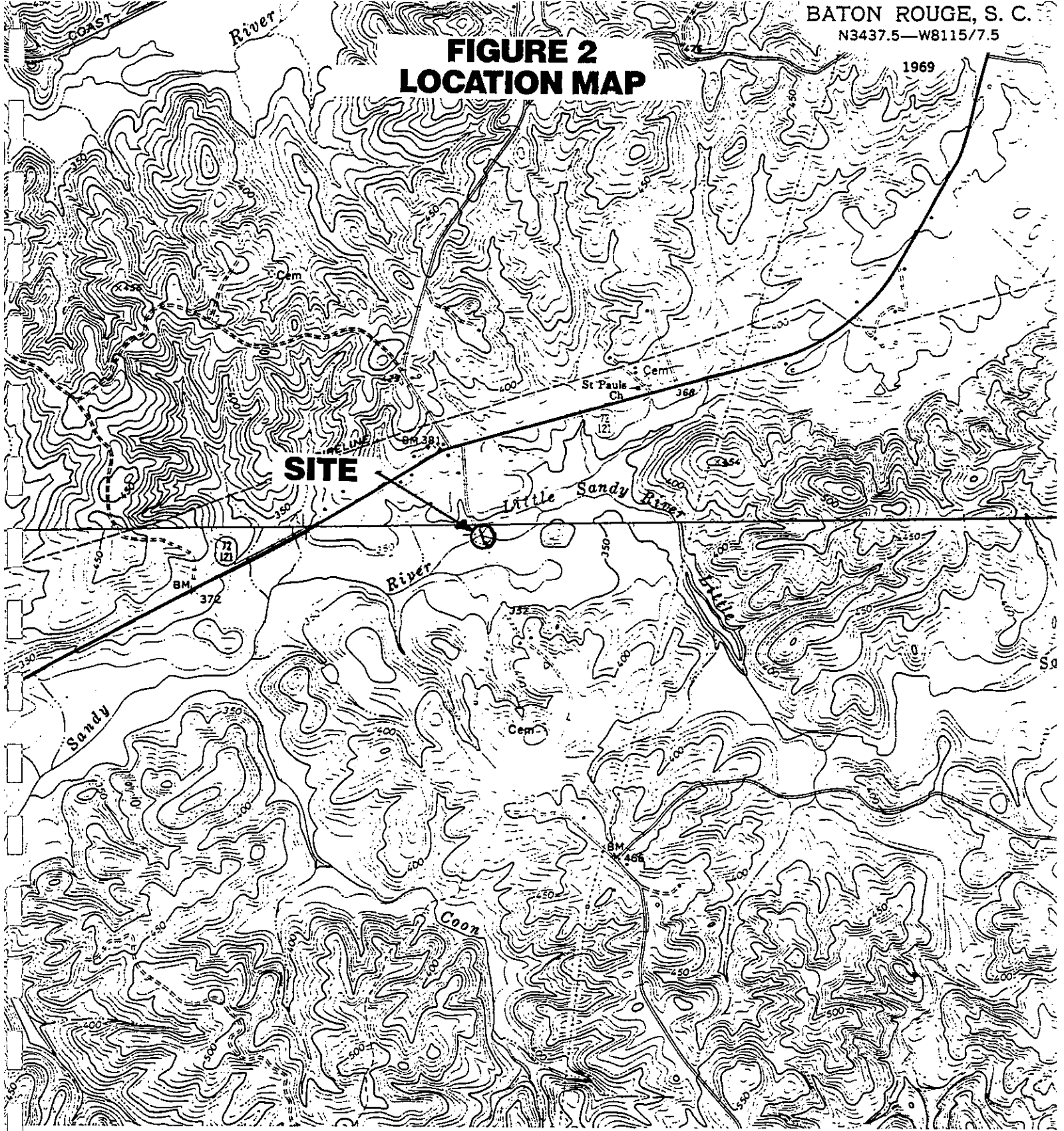
STREAM SIZE (SECT. 2.2.1)	Swamp (No Channel)	Small (<u><100 ft. wide</u>)	Medium (100-500 ft.)	Large (>500 ft.)
FLOW HABIT (SECT. 2.2.2)	Ephemeral	(Intermittent)	<u>Perennial but flashy</u>	Perennial
BED MATERIAL (SECT. 2.2.3)	Silt-clay	Silt	<u>Sand</u>	Gravel Cobble or boulder
VALLEY SETTING (SECT. 2.2.4)	 No valley; alluvial fan	 Low relief valley (<100 ft. or 30 m deep)	 Moderate relief (100-1000 ft. or 30-300 m)	 High relief (>1000 ft. or 300 m)
FLOOD PLAINS (SECT. 2.2.5)	 Little or none (<2X channel width)	 Narrow (2-10 channel width)	 Wide (>10X channel width)	
NATURAL LEVEES (SECT. 2.2.6)	 Little or None	 Mainly or Concave	 Well Developed on Both Banks	
APPARENT INCISION (SECT. 2.2.7)	 Not Incised		 Probably Incised	
CHANNEL BOUNDARIES (SECT. 2.2.8)	 Alluvial	 Semi-alluvial	 Non-alluvial	
TREE COVER ON BANKS (SECT. 2.2.8)	<30 percent of bankline	<u>50-90 percent</u>		> 90 percent
SINUOSITY (SECT. 2.2.9)	 Straight Sinuosity 1-1.05	 Sinuous (1.06-1.25)	 <u>Meandering</u> (1.25-2.0)	 Highly meandering (>2)
BRAIDED STREAMS (SECT. 2.2.10)	 Not braided (<5 percent)	 Locally braided (5-25 percent)	 Generally braided (> 25 percent)	
ANABRANCHED STREAMS (SECT. 2.2.11)	 Not anabranching (<5 percent)	 Locally anabranching (5-25 percent)	 Generally anabranching (> 25 percent)	
VARIABILITY OF WIDTH AND DEVELOPMENT OF BARS (SECT. 2.2.12)	 Narrow point bar	 Equiwidth	 Wider at banks	 Irregular point and lateral bars

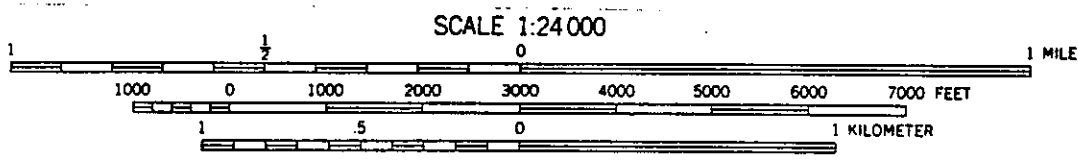
Figure 1. Geomorphic factors that affect stream stability (From HEC-20)

FIGURE 2 LOCATION MAP

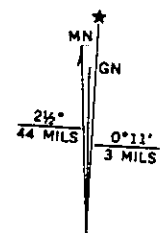
1969



1969



CONTOUR INTERVAL 10 FEET
DATUM IS MEAN SEA LEVEL



QUADRANGLE LOCATION

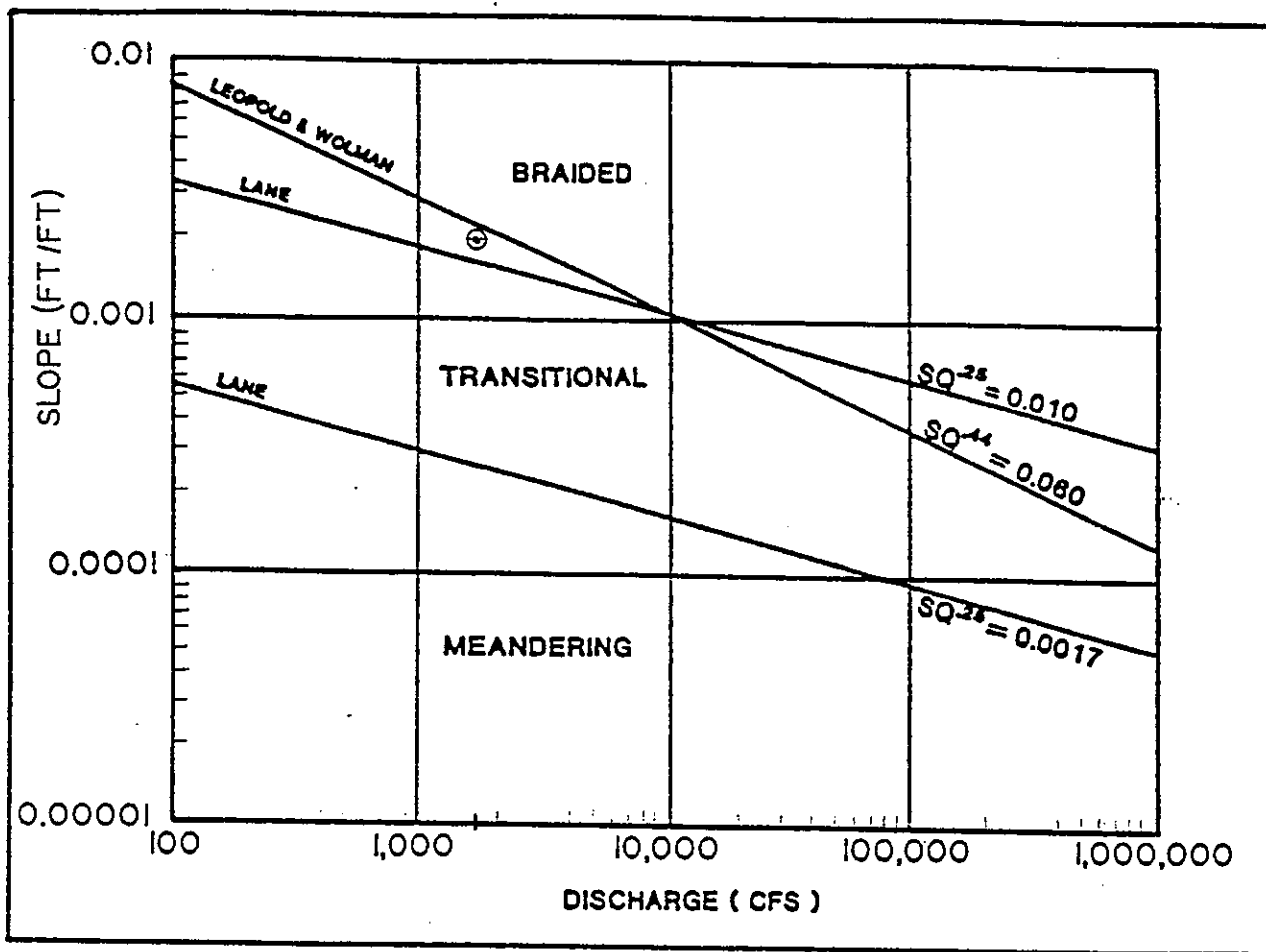


Figure 3. Slope-discharge relationship for braiding or meandering in sand bed streams (Figure 8 in HEC-20)

2.0 SCOUR ANALYSES

2.1 Flood History and Rainfall-Runoff Relations

The 1978 construction plans show a 1928 H.W. El. = 348.7 FT MSL. This water surface elevation is similar to the elevation that was calculated for the 500-year storm (348.2 FT MSL) by the WSPRO computer program.

The rainfall-runoff relations for the 34.1 sq. mi. drainage basin were obtained using the USGS Report 87-4096⁽⁵⁾ titled "Magnitude and Frequency of Floods in Rural and Urban Basins of North Carolina. The North Carolina equations were used because Chester County is in an area where the South Carolina equations are not accurate. The results of the regression equations for the Piedmont are listed below:

<u>Frequency (years)</u>	<u>Regression Equation Flowrate (cfs)</u>
2	1649
10	3489
25	4709
50	5756
100	6950
500	10322

2.2 Hydraulic Conditions

The hydraulic conditions were investigated using the WSPRO⁽³⁾ computer program and the bridge geometry measured in the field. The approach and exit cross sections were developed by combining the channel geometry from the bridge soundings and the flood plain geometry from a field survey along the upstream fill toe. The soundings were taken from the upstream face of the bridge along the entire bridge and along the downstream face of the bridge in the channel area.

The results of the computer analysis show that the bridge opening exceeds the limits that would be allowed for a new bridge at this site. The existing bridge creates 0.9 ft. of backwater and has a velocity of 5.7 fps in the bridge opening. Standard design practice for South Carolina is 1.0 ft. backwater and 5 fps maximum velocity for the 100-year event. In addition, the roadway is overtopped by storms greater than the 2-year event.

2.3 Bed and Bank Material Analysis

The field investigation visually identified the bed and bank materials as sand.

2.4 Watershed Sediment Yield Evaluation

The sediment yield for the watershed appears to be relatively stable as evidenced by the consistent stream bed elevation. A rural, well-vegetated drainage basin such as this one is generally characterized by relatively low sediment yields.

2.5 Rating Curve Shifts

Rating curve shifts have not been investigated at this time at the direction of SCDOT.

2.6 Scour Condition Evaluation

The bridge at this site was built in 1979. This current bridge replaced an old bridge at this site. The current bridge consists of a flat slab deck supported on 10" steel H piles with spill-thru abutments.

Scour conditions were evaluated using the WSPRO computer program and the scour equations presented in HEC-18⁽⁴⁾. The WSPRO computer output and the scour calculations are presented in Appendix 3. The results of the scour calculations have been summarized in Table 1.

The scour calculations resulted in depths of 1.6 and 3.0 feet for contraction and pier scour, respectively, during the overtopping storm.

The southern abutment scour for the overtopping storm was calculated to be 7.0 feet. Abutment scour depths for the 100- and 500-year storms were assumed to be half the overtopping depths.

The thalweg for this stream could easily shift to Bents 2,3 or 4 during the life of the structure. For this reason, the ground elevation at these bents was assumed to be the thalweg elevation.

The results of the scour calculations show that scour could expose a significant length of the piles. The pile tip elevations were from a SCDOT Pile Record Sheet and only the shortest piles were listed in Table 1. A geotechnical and structural review of the information will be needed to determine if the foundations are stable.

2.7 Recommendations

The scour calculations for the bridge show moderate scour depths. We recommend a structural engineer and a geotechnical engineer review the scour conditions at this site and decide if the stability of the bridge can be determined with the currently available information. The geotechnical engineer will need to determine if the calculated scour depths can be reduced due to erosion resistant material. The recommendations of the geotechnical engineer should then be given to a structural engineer for a structural stability analysis. The structural engineer should review the pile length data to determine the individual pile lengths.

The northern abutment is poorly protected with $D_{50}=10''$ stone and the southern abutment is not protected with stone. The quantity of stone is inadequate to protect the bridge from the calculated abutment scour depths of 4.7 to 7.0 feet. We recommend that the abutments be repaired to meet the current SCDOT standards for new construction by adding $D_{50}=12''$ stone to both abutments.

Table 1 - Remaining pile/footing penetration at piers/bents for Structure #127020100100 on Route S-201, crossing Little Sandy River in Chester County, South Carolina

Pier/bent ¹ number	Station ¹ (feet)	Pile tip ² elevation (feet)	Ground ³ elevation at pier/bent (feet)	Total ⁴ scour depth (feet)	Elevation of scour (feet)	Remaining ⁵ pile penetration (feet)
Overtopping discharge (2-yr) is 1,750 cfs - 100-year discharge is 6,950 cfs - 500-year discharge is 10,322 cfs						

Scour Information for Overtopping Storm (2-Yr):

1	492.5	318.8	NA	NA	NA	NA
2	522.5	320.2	338.3	11.6	326.7	6.5
3	552.5	323.5	338.3	4.6	333.7	10.2
4	582.5	320.3	338.3	9.3	329.0	8.8
5	612.5	324.9	NA	NA	NA	NA

Scour Information for 100-Year Storm:

1	492.5	318.8	NA	NA	NA	NA
2	522.5	320.2	338.3	9.1 $2.9 + 3.2$	329.2	9.0
3	552.5	323.5	338.3	5.6	332.7	9.2
4	582.5	320.3	338.3	8.0	330.3	10.1
5	612.5	324.9	NA	NA	NA	NA

Scour Information for 500-Year Storm:

1	492.5	318.8	NA	NA	NA	NA
2	522.5	320.2	338.3	8.1	330.2	10.0
3	552.5	323.5	338.3	4.6	333.7	10.1
4	582.5	320.3	338.3	7.0	331.3	11.0
5	612.5	324.9	NA	NA	NA	NA

¹Pier/bent number and station corresponds to South Carolina Department of Transportation bridge plans.

²Pile tip elevations obtained from SCDOT bridge plans. Shortest pile length listed.

³Thalweg elevation used since thalweg may shift during life of structure.

⁴Total scour depth is the sum of the contraction, pier/bent and abutment scour depths.

⁵A negative number signifies undermining of pile tip/footing.

APPENDIX 1
FIELD INFORMATION

HYDRAULIC DESIGN
AND
RISK ASSESSMENT FOR
BRIDGE/BRIDGE REPLACEMENT OVER
~~stream name~~ LITTLE SANDY RIVER
ROUTE/ROAD NUMBER S-201
FILE NO. _____ PROJECT NO. _____
CHESTER COUNTY, SOUTH CAROLINA
STR # 127020100100

DATE

Prepared By TPW
Checked By AYN

Signed and Sealed

(12/14/92)

PROJECT DESCRIPTION

County CHESTER Rt./Rd. No. S-201
 Stream LITTLE SANDY RIVER File No. 12.520 Project No. SBR-520
 PIN _____ Charge Code _____
 Project Engineer _____ Road Squad _____
 STR. # 127020160100

COMPARATIVE DATA

By TPW Date 2-2-95 Checked DVN Date 2-6-95

	* UPSTREAM	SITE
ROUTE/ROAD NO.	<u>S-16</u>	<u>S-201</u>
DIST. FROM NEW BR. (MI.)	<u>4.7</u>	<u>0</u>
DRAINAGE AREA (SQ. MI.)	<u>9.84</u>	<u>34.06</u>
ZONE	PIEDMONT	PIEDMONT
Q ₁₀ (CFS) +	<u>1528</u>	<u>3489</u>
Q ₂₅ +	<u>2088</u>	<u>4709</u>
Q ₅₀ +	<u>2568</u>	<u>5756</u>
Q ₁₀₀ +	<u>3128</u>	<u>6950</u>
Q ₅₀₀ +	<u>4768</u>	<u>10322</u>
BRIDGE LENGTH (FT.)	<u>150</u>	<u>120</u>
AVG. F. G. (FT.)	-	<u>348.328</u>
OPENING FURN. (SQ. FT.)	<u>1457.19</u>	<u>1617.62</u>
VELOCITY (FT./SEC.) _{1604r}	<u>2.15</u>	<u>11.25</u>
HIGH-WATER ELEV. (FT.)	-	<u>348.7</u>
HIGH-WATER DATE	-	<u>1928</u>
HIGH-WATER DEPTH	-	<u>10.2</u>
NORMAL-WATER ELEV. (FT.)	-	<u>340.10</u>
NORMAL-WATER DATE	<u>1-3-95</u>	<u>1-3-95</u>
NORMAL-WATER DEPTH (FT)	<u>0.5</u>	<u>1.6</u>
FILE/DOCKET/PROJ. NO.	-	<u>12.520/SBR-520</u>
LOCATION OF PLANS	<u>SCDOT</u>	<u>SCDOT</u>
DATUM/DATUM TIE	-	-
FLOODWAY MAP	-	-

* NOT GOOD FOR COMPARISON: THE DRAINAGE AREA IS LESS THAN HALF THE DRAINAGE AREA OF THE BRIDGE SITE @ S-201.

+ The South Carolina Regression equations are not accurate in Chester County therefore the regression equations from Magnitude and Frequency of Floods in Rural and Urban North Carolina (U.S. Geological Survey Report 87-4096) were used at the request of SCDOT.

FIELD INSPECTION OF SCOUR BRIDGES (USGS)

COUNTY CHESTER Rt./Rd. No. S-201

STREAM NAME LITTLE SANDY RIVER STRUCTURE # 127020100100

CONSULTANT RWA INSPECTED BY DOUG K. / TONIA W.

Date 1-3-95

LENGTH 120 Ft. WIDTH 33.3 Ft. MAX.SPAN LENGTH 30 Ft.

Alignment (Tangent)/Curved Bridge Skewed Yes/No Angle _____

End Abutment Type SPILL THROUGH

RipRap on fills ? Yes/No Condition NORTHERN = POOR D50 = 10"
NORTHERN SOUTHERN NONE ON SOUTHERN

Superstructure Type FLAT SLAB

Substructure Type 10" STEEL H PILES

Debris accumulations on bridge. Yes/No

Percent channel blocked horizontal 10

Percent channel blocked vertical 10

Guide Banks (Spur Dikes) Yes/No

Channel Spanned ? Yes/No

If No, number of bents in or near channel banks 2

Is the bridge crossing located in or near a channel bend ?

Yes/No

Is the bridge located on a tidal stream ? Yes/No

Are there any visible scour holes ? Yes/No

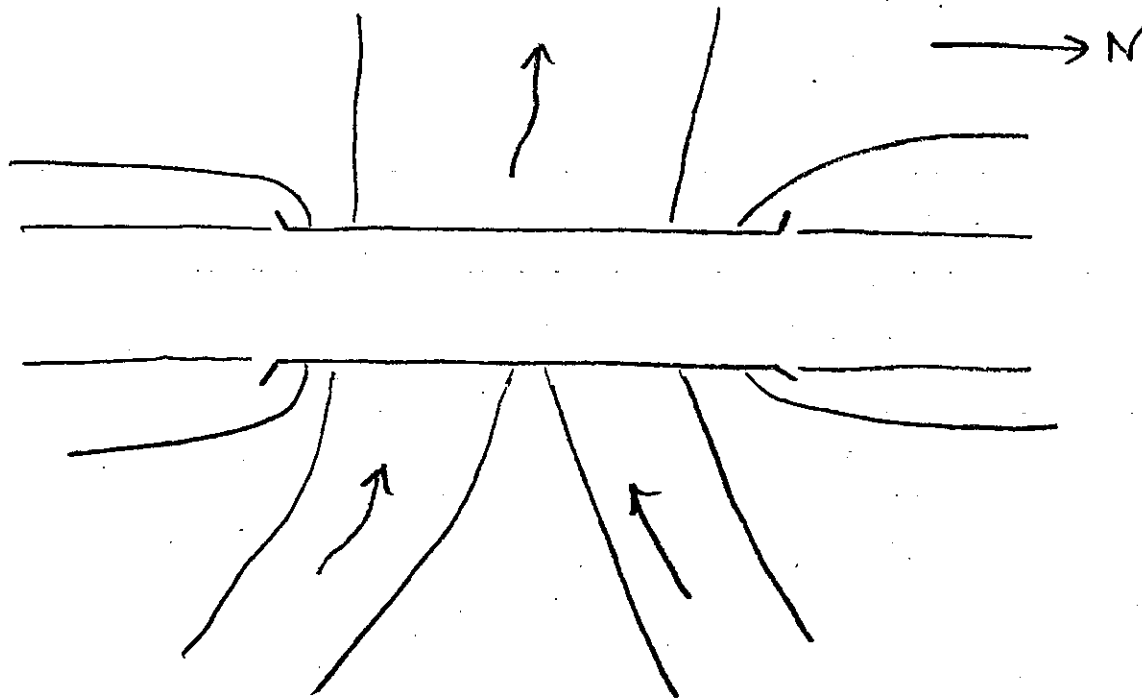
If Yes, locate scour holes on sketch.

Bank Condition: stable ✓, scalloped _____

Rock Present ? Yes/No Boulders, Bed Rock, Shoals, etc.

If Yes, what type ? _____

Draw Sketch of Bridge and Stream Below
(Show north arrow and direction of flow)



Site Characteristics

General Topography ROLLING HILLS
 Stream Type (circle one) (Straight) Braided, or Meandering
 Are Channel banks Stable? Yes/No If No, Describe vegetation - dense
woods upstream on the North side and pasture-type grass on the remaining banks
 Are there any Hydraulic Controls Upstream or Downstream? Yes/No
 Describe _____

Soil type SAND Exposed Rock Yes (No) If so, give description and location _____

Describe potential for drift MODERATE

Give description and location of any structures or other property that could be damaged by backwater NONE

Describe any other features that might affect or be affected by the hydraulic performance of the proposed bridge ROADWAY LIKELY OVERTOPPED BEFORE BRIDGE IS OVERTOPPED

Mannings "n" Values

Channel

		$n = (n_b + n_1 + n_2 + n_3 + n_4) m$									
n_b -- Base n for soil	<table border="0" style="width: 100%;"> <tr><td><u>Earth</u></td><td style="text-align: right;"><u>.020</u></td></tr> <tr><td>Rock Cut</td><td style="text-align: right;">.025</td></tr> <tr><td>Fine Gravel</td><td style="text-align: right;">.024</td></tr> <tr><td>Course Gravel</td><td style="text-align: right;">.028</td></tr> </table>	<u>Earth</u>	<u>.020</u>	Rock Cut	.025	Fine Gravel	.024	Course Gravel	.028		UPSTREAM: PASTURE DOWNSTREAM SOUTH: PASTURE DOWNSTREAM NORTH: WOODS (moderate to dense)
<u>Earth</u>	<u>.020</u>										
Rock Cut	.025										
Fine Gravel	.024										
Course Gravel	.028										
n_1 -- Degree of Irregularity	<table border="0" style="width: 100%;"> <tr><td>Smooth</td><td style="text-align: right;">.000</td></tr> <tr><td><u>Minor</u></td><td style="text-align: right;"><u>.001-.005</u></td></tr> <tr><td>Moderate</td><td style="text-align: right;">.006-.010</td></tr> <tr><td>Severe</td><td style="text-align: right;">.011-.020</td></tr> </table>	Smooth	.000	<u>Minor</u>	<u>.001-.005</u>	Moderate	.006-.010	Severe	.011-.020		
Smooth	.000										
<u>Minor</u>	<u>.001-.005</u>										
Moderate	.006-.010										
Severe	.011-.020										
n_2 -- Variations of Channel Cross Sections	<table border="0" style="width: 100%;"> <tr><td>Gradual</td><td style="text-align: right;">.000</td></tr> <tr><td><u>Alternating</u></td><td></td></tr> <tr><td><u>occasionally</u></td><td style="text-align: right;"><u>.001-.005</u></td></tr> <tr><td>Frequently</td><td style="text-align: right;">.010-.015</td></tr> </table>	Gradual	.000	<u>Alternating</u>		<u>occasionally</u>	<u>.001-.005</u>	Frequently	.010-.015		
Gradual	.000										
<u>Alternating</u>											
<u>occasionally</u>	<u>.001-.005</u>										
Frequently	.010-.015										
n_3 -- Relative Effect of Obstructions	<table border="0" style="width: 100%;"> <tr><td><u>Negligible</u></td><td style="text-align: right;"><u>.000-.004</u></td></tr> <tr><td>Minor</td><td style="text-align: right;">.010-.015</td></tr> <tr><td>Appreciable</td><td style="text-align: right;">.020-.030</td></tr> <tr><td>Severe</td><td style="text-align: right;">.040-.060</td></tr> </table>	<u>Negligible</u>	<u>.000-.004</u>	Minor	.010-.015	Appreciable	.020-.030	Severe	.040-.060		
<u>Negligible</u>	<u>.000-.004</u>										
Minor	.010-.015										
Appreciable	.020-.030										
Severe	.040-.060										

n ₄ -- Vegetation	Low	.002-.010
	Medium	.010-.025
	High	.025-.050
	Very High	.050-.100
m -- Degree of Meandering	Minor	1.00
	Appreciable	1.15
	Severe	1.30

Field Observations
for Channel

Channel Depth	n _b	n ₁	n ₂	n ₃	n ₄	m	Computed n
	.02	.001	.005	.002	.003	1.00	.031

Mannings "n"
For Over Bank Areas

$$n = n_b + n_1 + n_3 + n_4$$

n _b -- Base n for soil	Earth	.020
	Rock Cut	.025
	Fine Gravel	.024
	Course Gravel	.028
n ₁ -- Degree of Irregularity	Smooth	.000
	Minor	.001-.005 ←
	Moderate	.006-.010
	Severe	.011-.020
n ₃ -- Effect of Obstructions	Negligible	.000-.004
	Minor	.005-.019
	Appreciable	.020-.030
n ₄ -- Amount of Vegetation	Small	.001-.010
	Medium	.011-.025
	Large	.025-.Very Large

STREAM BED SOUNDINGS

BRIDGE NO. 1251020100100 COUNTY CHESTER DATE 1-3-95 BY DOUG M. / TONIA W.

9-201 L. Sandy River

RECORD SOUNDINGS FROM TOP OF RAIL. OTHER LOCATION IF NEEDED: _____

DISTANCE H. W. MARK TO TOP OF RAIL 4.8 LOCATION H. W. MARK NEAR BENT 3

DOWNSTREAM

PGL to Top of Rail = 2.4

UPSTREAM

STATION	SOUNDING	DESCRIPTION	STATION	SOUNDING	DESCRIPTION
117.6	5.6		-	4.1	NORTH Lower Chord
119.7		SOUTH FF	1.3	6.0	
			5.8	6.7	TOP OF BANK
			10.4	9.1	
— DOWNSTREAM —			17.3	11.1	EDGE WATER
24.1	11.1	NORTH EDGE WATER	20.3	11.9	
32.4	11.1	EDGE WATER	25.0	12.0	
39.0	9.2		30.0	11.9	BENT 2
48.7	11.1	EDGE WATER	34.1	12.7	
56.8	12.0		37.1	11.1	EDGE WATER ←
64.5	12.2		43.9	8.0	TOP OF BANK
76.7	11.4		52.9	7.9	
82.2	10.9	EDGE WATER	59.8	7.8	BENT 3
			64.9	7.6	TOP OF BANK
			68.2	10.1	BOTTOM BANK
			75.8	10.8	EDGE WATER
			81.0	11.5	
			84.0	11.7	
			89.7	10.9	BENT 4
			94.7	10.7	EDGE WATER
			102.3	8.1	TOP BANK
			110.1	6.6	

COMPARATIVE BRIDGE SITE INSPECTION FORM

County Chester Rt/Rd No. S-116
Stream LITTLE SANDY RIVER Measured bridge length 150

Maximum span length 90 Superstructure type FLAT SLAB & STEEL GIRDERS

Substructure type DRILLED SHAFT End Abutment type Spillthrough

Rip-rap present? (Yes)/No Condition EXCELLENT

Stream type (circle one) (Straight), Braided, Meander, or Anabranching. Alluvial or (Rock) (circle)

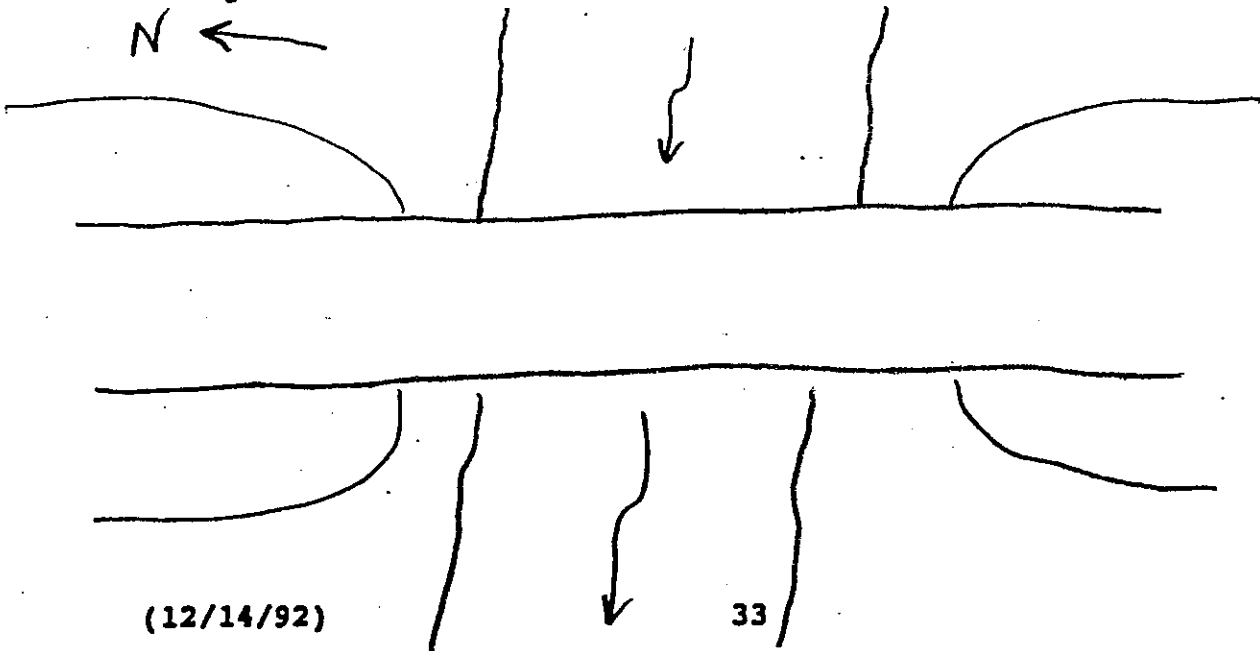
Any visible signs of scour problems (describe) NO

Are banks stable (describe) yes, vegetated

Debris blockage; Percent of channel blocked horizontally NONE
vertically . Describe other signs of debris

Any other problems NONE

Draw sketch and indicate problem areas. On sketch indicate location of woods, fields and other land uses in the vicinity of bridge. Show north arrow and direction of flow.



Flood History

Local resident's name: Local Resident in passing pick-up truck
address:

Phone #:

Period of knowledge: 50 Yrs.

High water mark location: water gets to bottom of bridge 2

Date of occurrence: or 3 times a year. Water flows

Frequency of flooding: over road 3 or 4 times a year.

APPENDIX 2
PHOTOGRAPHS

S-201 OVER LITTLE SANDY RIVER
Chester County



(1) LOOKING NORTH ALONG ALIGNMENT.



(2) LOOKING UPSTREAM ALONG NORTHERN BRANCH.

S-201 OVER LITTLE SANDY RIVER
Chester County



(3) LOOKING UPSTREAM ALONG SOUTHERN BRANCH.



(4) LOOKING DOWNSTREAM.

S-201 OVER LITTLE SANDY RIVER
Chester County



(5) UPSTREAM FACE LOOKING NORTH.



(6) DOWNSTREAM FACE LOOKING NORTH.

S-201 OVER LITTLE SANDY RIVER
Chester County



(7) NORTHERN END BENT (#5).



(8) LOOKING SOUTH ALONG DOWNSTREAM SIDE OF BRIDGE.

S-201 OVER LITTLE SANDY RIVER
Chester County



(9) LOOKING UPSTREAM AT BENT 2.



(10) LOOKING UPSTREAM AT SOUTHERN END BENT (#1).

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FEDERAL HIGHWAY ADMINISTRATION - U. S. GEOLOGICAL SURVEY
MODEL FOR WATER-SURFACE PROFILE COMPUTATIONS
(Input modified to free format by GKY&A 01/92)

*** RUN DATE & TIME: 02-15-95 09:28

T1 STR. NO. 127020100100 CHESTER CO.

T2 S-201 OVER LITTLE SANDY RIVER

*F

Q 1750 6950 10322

** Q-DATA FOR SEC-ID, ISEQ = 1
SK 0.0019 0.0019 0.0019

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FEDERAL HIGHWAY ADMINISTRATION - U. S. GEOLOGICAL SURVEY
MODEL FOR WATER-SURFACE PROFILE COMPUTATIONS
(Input modified to free format by GKY&A 01/92)

STR. NO. 127020100100 CHESTER CO.
S-201 OVER LITTLE SANDY RIVER

*** RUN DATE & TIME: 02-15-95 09:28

** START PROCESSING CROSS SECTION - "1 "

XT 1 1000

GR	252.5	370.0	302.5	360.0	352.5	350.0	982.5	344.2
GR	1012.5	343.9	1042.5	343.8	1072.5	343.7	1102.5	343.7
GR	1132.5	343.5	1162.5	343.4	1192.5	343.3	1222.5	343.5
GR	1252.5	343.3	1282.5	343.1	1312.5	343.4	1342.5	342.9
GR	1372.5	343.1	1402.5	343.5	1432.5	343.6	1462.5	344.0
GR	1492.5	344.5	1494.9	344.5	1502.4	343.5	1510.2	342.1
GR	1517.8	339.6	1522.8	339.5	1528.5	338.7	1531.5	339.0
GR	1536.7	339.7	1544.3	340.5	1547.6	343.0	1552.7	342.9
GR	1559.6	342.9	1568.6	342.9	1575.4	339.8	1578.4	338.3
GR	1582.5	339.1	1587.5	339.1	1592.2	339.2	1595.2	340.1
GR	1602.1	342.1	1606.7	344.6	1612.5	344.7	1642.5	344.2
GR	1672.5	344.9	1702.5	346.1	1732.5	348.5	1762.5	351.1
GR	1862.5	360.0						

** FINISH PROCESSING CROSS SECTION - "1 "

*** TEMPLATE CROSS SECTION "1 " SAVED INTERNALLY.

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FEDERAL HIGHWAY ADMINISTRATION - U. S. GEOLOGICAL SURVEY
MODEL FOR WATER-SURFACE PROFILE COMPUTATIONS
(Input modified to free format by GKY&A 01/92)

STR. NO. 127020100100 CHESTER CO.
S-201 OVER LITTLE SANDY RIVER

*** RUN DATE & TIME: 02-15-95 09:28

*** START PROCESSING CROSS SECTION - "EXIT "

XS EXIT 880 0 * * 0.0019

GT

N 0.047 0.031 0.15 0.031 0.15

SA 1510.2 1547.6 1568.6 1606.7

*** FINISH PROCESSING CROSS SECTION - "EXIT "

*** CROSS SECTION "EXIT " WRITTEN TO DISK, RECORD NO. = 1

-- DATA SUMMARY FOR SECID "EXIT " AT SRD = 880. ERR-CODE = 0

SKEW IHFNO VSLOPE EK CK
.0 0. .0019 .50 .00

X-Y COORDINATE PAIRS (NGP = 49):

X	Y	X	Y	X	Y	X	Y
252.5	369.77	302.5	359.77	352.5	349.77	982.5	343.97
1012.5	343.67	1042.5	343.57	1072.5	343.47	1102.5	343.47
1132.5	343.27	1162.5	343.17	1192.5	343.07	1222.5	343.27
1252.5	343.07	1282.5	342.87	1312.5	343.17	1342.5	342.67
1372.5	342.87	1402.5	343.27	1432.5	343.37	1462.5	343.77
1492.5	344.27	1494.9	344.27	1502.4	343.27	1510.2	341.87
1517.8	339.37	1522.8	339.27	1528.5	338.47	1531.5	338.77
1536.7	339.47	1544.3	340.27	1547.6	342.77	1552.7	342.67
1559.6	342.67	1568.6	342.67	1575.4	339.57	1578.4	338.07
1582.5	338.87	1587.5	338.87	1592.2	338.97	1595.2	339.87
1602.1	341.87	1606.7	344.37	1612.5	344.47	1642.5	343.97
1672.5	344.67	1702.5	345.87	1732.5	348.27	1762.5	350.87
1862.5	359.77						

X-Y MAX-MIN POINTS:

XMIN Y X YMIN XMAX Y X YMAX
252.5 369.77 1578.4 338.07 1862.5 359.77 252.5 369.77

SUBAREA BREAKPOINTS (NSA = 5):

1510. 1548. 1569. 1607.

ROUGHNESS COEFFICIENTS (NSA = 5):

.047 .031 .150 .031 .150

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FEDERAL HIGHWAY ADMINISTRATION - U. S. GEOLOGICAL SURVEY
MODEL FOR WATER-SURFACE PROFILE COMPUTATIONS
(Input modified to free format by GKY&A 01/92)

STR. NO. 127020100100 CHESTER CO.
S-201 OVER LITTLE SANDY RIVER

*** RUN DATE & TIME: 02-15-95 09:28

** START PROCESSING CROSS SECTION - "FULLV"

XS FULLV 1000 0 * * 0.0019

GT

N 0.047 0.031 0.15 0.031 0.15

SA 1510.2 1547.6 1568.6 1606.7

** FINISH PROCESSING CROSS SECTION - "FULLV"

*** CROSS SECTION "FULLV" WRITTEN TO DISK, RECORD NO. = 2

-- DATA SUMMARY FOR SECID "FULLV" AT SRD = 1000. ERR-CODE = 0

SKEW IHFNO VSLOPE EK CK
.0 0. .0019 .50 .00

X-Y COORDINATE PAIRS (NGP = 49):

X	Y	X	Y	X	Y	X	Y
252.5	370.00	302.5	360.00	352.5	350.00	982.5	344.20
1012.5	343.90	1042.5	343.80	1072.5	343.70	1102.5	343.70
1132.5	343.50	1162.5	343.40	1192.5	343.30	1222.5	343.50
1252.5	343.30	1282.5	343.10	1312.5	343.40	1342.5	342.90
1372.5	343.10	1402.5	343.50	1432.5	343.60	1462.5	344.00
1492.5	344.50	1494.9	344.50	1502.4	343.50	1510.2	342.10
1517.8	339.60	1522.8	339.50	1528.5	338.70	1531.5	339.00
1536.7	339.70	1544.3	340.50	1547.6	343.00	1552.7	342.90
1559.6	342.90	1568.6	342.90	1575.4	339.80	1578.4	338.30
1582.5	339.10	1587.5	339.10	1592.2	339.20	1595.2	340.10
1602.1	342.10	1606.7	344.60	1612.5	344.70	1642.5	344.20
1672.5	344.90	1702.5	346.10	1732.5	348.50	1762.5	351.10
1862.5	360.00						

X-Y MAX-MIN POINTS:

XMIN	Y	X	YMIN	XMAX	Y	X	YMAX
252.5	370.00	1578.4	338.30	1862.5	360.00	252.5	370.00

SUBAREA BREAKPOINTS (NSA = 5):

1510. 1548. 1569. 1607.

ROUGHNESS COEFFICIENTS (NSA = 5):

.047 .031 .150 .031 .150

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FEDERAL HIGHWAY ADMINISTRATION - U. S. GEOLOGICAL SURVEY
MODEL FOR WATER-SURFACE PROFILE COMPUTATIONS
(Input modified to free format by GKY&A 01/92)

STR. NO. 127020100100 CHESTER CO.
S-201 OVER LITTLE SANDY RIVER

*** RUN DATE & TIME: 02-15-95 09:28

*** START PROCESSING CROSS SECTION - "BRID "

BR BRID 1000 346.5 0

GR 1492.5 345.9 1494.9 344.5 1502.4 343.5 1510.2 342.1

GR 1517.8 339.6 1522.8 339.5 1528.5 338.7 1531.5 339.0

GR 1536.7 339.7 1544.3 340.5 1547.6 343.0 1552.7 342.9

GR 1559.6 342.9 1568.6 342.9 1575.4 339.8 1578.4 338.3

GR 1582.5 339.1 1587.5 339.1 1592.2 339.2 1595.2 340.1

GR 1602.1 342.1 1606.7 344.6 1611.2 345.3 1612.5 347.3

GR 1492.5 345.9

N 0.04 0.031 0.04 0.031 0.04

SA 1510.2 1547.6 1568.6 1606.7

CD 3 33.3 2.000 348.3 0 0 0

PW 1 339.2 0.833 339.5 0.833 339.5 1.666 342.9 1.666 342.9 2.5

346.3 2.5 346.3 1.666 346.6 1.666 346.6 0.833 347 0.833

347 0

HP 1 BRID 343.9 * 343.9

** FINISH PROCESSING CROSS SECTION - "BRID "

** CROSS SECTION "BRID " WRITTEN TO DISK, RECORD NO. = 3

--- DATA SUMMARY FOR SECID "BRID " AT SRD = 1000. ERR-CODE = 0

SKEW IHFNO VSLOPE EK CK
.0 0. .0019 .50 .00

-Y COORDINATE PAIRS (NGP = 25):

X	Y	X	Y	X	Y	X	Y
1492.5	345.90	1494.9	344.50	1502.4	343.50	1510.2	342.10
1517.8	339.60	1522.8	339.50	1528.5	338.70	1531.5	339.00
1536.7	339.70	1544.3	340.50	1547.6	343.00	1552.7	342.90
1559.6	342.90	1568.6	342.90	1575.4	339.80	1578.4	338.30
1582.5	339.10	1587.5	339.10	1592.2	339.20	1595.2	340.10
1602.1	342.10	1606.7	344.60	1611.2	345.30	1612.5	347.30
1492.5	345.90						

X-Y MAX-MIN POINTS:

XMIN	Y	X	YMIN	XMAX	Y	X	YMAX
1492.5	345.90	1578.4	338.30	1612.5	347.30	1612.5	347.30

SUBAREA BREAKPOINTS (NSA = 5):

ROUGHNESS COEFFICIENTS (NSA = 5):

.040 .031 .040 .031 .040

BRIDGE PARAMETERS:

BRTYPE	BRWIDTH	LSEL	USERCD	EMBSS	EMBELV	ABSLPL	ABSLPR
3	33.3	346.50	*****	2.00	348.30	*****	*****

PIER DATA: NPW = 11 PPCD = 1.

PELV	PWDTH	PELV	PWDTH	PELV	PWDTH	PELV	PWDTH
339.20	.8	339.50	.8	339.50	1.7	342.90	1.7
342.90	2.5	346.30	2.5	346.30	1.7	346.60	1.7
346.60	.8	347.00	.8	347.00	.0		

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FEDERAL HIGHWAY ADMINISTRATION - U. S. GEOLOGICAL SURVEY
MODEL FOR WATER-SURFACE PROFILE COMPUTATIONS
(Input modified to free format by GKY&A 01/92)

STR. NO. 127020100100 CHESTER CO.
S-201 OVER LITTLE SANDY RIVER

*** RUN DATE & TIME: 02-15-95 09:28

CROSS-SECTION PROPERTIES: ISEQ = 3; SECID = BRID ; SRD = 1000.

WSEL	SA#	AREA	K	TOPW	WETP	ALPH	LEW	REW	QCR
	1	9.	304.	11.	11.				48.
	2	147.	17185.	37.	39.				1655.
	3	21.	766.	21.	21.				117.
	4	135.	14825.	37.	39.				1460.
343.90		312.	33081.	106.	110.	1.12	1499.	1605.	2870.

HP

1 BRID 347.19

* 347.19



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FEDERAL HIGHWAY ADMINISTRATION - U. S. GEOLOGICAL SURVEY
MODEL FOR WATER-SURFACE PROFILE COMPUTATIONS
(Input modified to free format by GKY&A 01/92)

STR. NO. 127020100100 CHESTER CO.
S-201 OVER LITTLE SANDY RIVER

*** RUN DATE & TIME: 02-15-95 09:28

CROSS-SECTION PROPERTIES: ISEQ = 3; SECID = BRID ; SRD = 1000.

WSEL	SA#	AREA	K	TOPW	WETP	ALPH	LEW	REW	QCR
	1	42.	1730.	0.	36.				0.
	2	238.	24400.	0.	76.				0.
	3	79.	4467.	0.	42.				0.
	4	253.	27336.	4.	75.				11957.
	5	11.	585.	6.	7.				89.
347.19		622.	58519.	<u>119.</u>	236.	1.15	1493.	1612.	26831.

1612 - 1493 = 119 ft

HP

1 BRID 347.07

* 347.07



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FEDERAL HIGHWAY ADMINISTRATION - U. S. GEOLOGICAL SURVEY
MODEL FOR WATER-SURFACE PROFILE COMPUTATIONS
(Input modified to free format by GKY&A 01/92)

STR. NO. 127020100100 CHESTER CO.
S-201 OVER LITTLE SANDY RIVER

*** RUN DATE & TIME: 02-15-95 09:28

CROSS-SECTION PROPERTIES: ISEQ = 3; SECID = BRID ; SRD = 1000.

WSEL	SA#	AREA	K	TOPW	WETP	ALPH	LEW	REW	QCR
	1	42.	1730.	0.	36.				0.
	2	238.	24400.	0.	76.				0.
	3	79.	4467.	0.	42.				0.
	4	251.	29964.	14.	64.				6068.
	5	11.	534.	6.	7.				82.
347.07		621.	61096.	20.	225.	1.18	1493.	1612.	18229.

HP

2 BRID 343.9 * 343.9 1750



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FEDERAL HIGHWAY ADMINISTRATION - U. S. GEOLOGICAL SURVEY
MODEL FOR WATER-SURFACE PROFILE COMPUTATIONS
(Input modified to free format by GKY&A 01/92)

STR. NO. 127020100100 CHESTER CO.
S-201 OVER LITTLE SANDY RIVER
*** RUN DATE & TIME: 02-15-95 09:28

VELOCITY DISTRIBUTION: ISEQ = 3; SECID = BRID ; SRD = 1000.

	WSEL	LEW	REW	AREA	K	Q	VEL
	343.90	1499.4	1605.4	311.6	33081.	1750.	5.62
X STA.	1499.4		1516.2	1519.4	1522.4	1525.2	1527.7
A(I)		25.8		13.3	13.0	12.9	12.3
V(I)		3.39		6.57	6.71	6.79	7.10
X STA.	1527.7		1530.1	1532.6	1535.5	1538.9	1542.5
A(I)		12.2		12.4	13.1	14.2	13.8
V(I)		7.18		7.04	6.69	6.16	6.36
X STA.	1542.5		1567.4	1575.7	1578.5	1580.9	1583.5
A(I)		32.8		19.8	13.9	12.8	12.5
V(I)		2.67		4.42	6.29	6.83	6.99
X STA.	1583.5		1586.0	1588.6	1591.3	1594.1	1605.4
A(I)		12.2		12.5	12.7	12.7	26.6
V(I)		7.16		6.99	6.91	6.88	3.29

HP

2 BRID 347.19

* 347.19 3455



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FEDERAL HIGHWAY ADMINISTRATION - U. S. GEOLOGICAL SURVEY
MODEL FOR WATER-SURFACE PROFILE COMPUTATIONS
(Input modified to free format by GKY&A 01/92)

STR. NO. 127020100100 CHESTER CO.
S-201 OVER LITTLE SANDY RIVER
*** RUN DATE & TIME: 02-15-95 09:28

VELOCITY DISTRIBUTION: ISEQ = 3; SECID = BRID ; SRD = 1000.

	WSEL	LEW	REW	AREA	K	Q	VEL
	347.19	1492.5	1612.4	622.3	58519.	3455.	5.55
STA.	1492.5		1513.6	1518.6	1522.8	1526.5	1529.9
A(I)		57.4		30.4	27.5	26.2	25.4
V(I)		3.01		5.68	6.28	6.59	6.81
X STA.	1529.9		1533.4	1537.4	1541.8	1548.6	1562.5
A(I)		26.2		27.4	28.5	34.5	51.8
V(I)		6.60		6.32	6.07	5.01	3.33
X STA.	1562.5		1572.6	1576.8	1579.8	1582.9	1586.0
A(I)		42.6		28.7	24.7	25.0	24.6
V(I)		4.06		6.02	6.99	6.90	7.03
STA.	1586.0		1589.1	1592.3	1595.8	1600.5	1612.4
A(I)		24.6		24.7	25.7	29.3	37.2
V(I)		7.01		6.99	6.72	5.89	4.65

HP

2 BRID 347.07

* 347.07 3705



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FEDERAL HIGHWAY ADMINISTRATION - U. S. GEOLOGICAL SURVEY
MODEL FOR WATER-SURFACE PROFILE COMPUTATIONS
(Input modified to free format by GKY&A 01/92)

STR. NO. 127020100100 CHESTER CO.
S-201 OVER LITTLE SANDY RIVER

*** RUN DATE & TIME: 02-15-95 09:28

VELOCITY DISTRIBUTION: ISEQ = 3; SECID = BRID ; SRD = 1000.

WSEL	LEW	REW	AREA	K	Q	VEL
347.07	1492.5	1612.4	620.6	61096.	3705.	5.97

STA.	1492.5	1513.8	1518.9	1523.2	1527.1	1530.6
A(I)		58.7	31.0	28.4	27.6	26.7
V(I)		3.15	5.97	6.51	6.72	6.94

X STA.	1530.6	1534.4	1538.7	1543.5	1554.4	1568.3
A(I)		27.7	28.6	30.7	45.1	53.0
V(I)		6.69	6.48	6.04	4.11	3.49

X STA.	1568.3	1574.8	1578.5	1581.6	1584.8	1588.1
A(I)		34.3	28.0	25.7	25.9	25.6
V(I)		5.41	6.61	7.20	7.15	7.24

X STA.	1588.1	1591.3	1594.4	1597.3	1601.0	1612.4
A(I)		25.5	23.3	20.0	21.5	33.3
V(I)		7.27	7.95	9.28	8.60	5.57

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FEDERAL HIGHWAY ADMINISTRATION - U. S. GEOLOGICAL SURVEY
MODEL FOR WATER-SURFACE PROFILE COMPUTATIONS
(Input modified to free format by GKY&A 01/92)

STR. NO. 127020100100 CHESTER CO.
S-201 OVER LITTLE SANDY RIVER

*** RUN DATE & TIME: 02-15-95 09:28

** START PROCESSING CROSS SECTION - "ROAD "

XR ROAD 1016.6 33.3 1

GR 252.5 370 302.5 360 352.5 350 1340 344.7

GR 1492.5 347.7 1612.5 349.0 1740 346.6 1867.5 348.9

*** FINISH PROCESSING CROSS SECTION - "ROAD "

*** NO ROUGHNESS DATA INPUT, WILL PROPAGATE FROM PREVIOUS CROSS SECTION.

** CROSS SECTION "ROAD " WRITTEN TO DISK, RECORD NO. = 4

--- DATA SUMMARY FOR SECID "ROAD " AT SRD = 1017. ERR-CODE = 0

SKEW	IHFNO	VSLOPE	EK	CK
.0	0.	.0019	.50	.00

-Y COORDINATE PAIRS (NGP = 8):

X	Y	X	Y	X	Y	X	Y
252.5	370.00	302.5	360.00	352.5	350.00	1340.0	344.70
1492.5	347.70	1612.5	349.00	1740.0	346.60	1867.5	348.90

X-Y MAX-MIN POINTS:

XMIN	Y	X	YMIN	XMAX	Y	X	YMAX
252.5	370.00	1340.0	344.70	1867.5	348.90	252.5	370.00

SUBAREA BREAKPOINTS (NSA = 5):

1510. 1548. 1569. 1607.

ROUGHNESS COEFFICIENTS (NSA = 5):

.047 .031 .150 .031 .150

ROAD GRADE DATA: IPAVE RDWID USERCF

1. 33.3 *****

BRIDGE PROJECTION DATA: XREFLT XREFRT FDSTLT FDSTRT

***** ***** ***** *****

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FEDERAL HIGHWAY ADMINISTRATION - U. S. GEOLOGICAL SURVEY
MODEL FOR WATER-SURFACE PROFILE COMPUTATIONS
(Input modified to free format by GKY&A 01/92)

STR. NO. 127020100100 CHESTER CO.
S-201 OVER LITTLE SANDY RIVER
*** RUN DATE & TIME: 02-15-95 09:28

** START PROCESSING CROSS SECTION - "APPR "
AS APPR 1153.3 0 * * 0.0019

GT
N 0.047 0.031 0.15 0.031 0.15
SA 1510.2 1547.6 1568.6 1606.7
HP 1 APPR 344.69 * 344.69

*** FINISH PROCESSING CROSS SECTION - "APPR "

** CROSS SECTION "APPR " WRITTEN TO DISK, RECORD NO. = 5

--- DATA SUMMARY FOR SECID "APPR " AT SRD = 1153. ERR-CODE = 0

SKEW IHFNO VSLOPE EK CK
.0 0. .0019 .50 .00

-Y COORDINATE PAIRS (NGP = 49):

X	Y	X	Y	X	Y	X	Y
252.5	370.29	302.5	360.29	352.5	350.29	982.5	344.49
1012.5	344.19	1042.5	344.09	1072.5	343.99	1102.5	343.99
1132.5	343.79	1162.5	343.69	1192.5	343.59	1222.5	343.79
1252.5	343.59	1282.5	343.39	1312.5	343.69	1342.5	343.19
1372.5	343.39	1402.5	343.79	1432.5	343.89	1462.5	344.29
1492.5	344.79	1494.9	344.79	1502.4	343.79	1510.2	342.39
1517.8	339.89	1522.8	339.79	1528.5	338.99	1531.5	339.29
1536.7	339.99	1544.3	340.79	1547.6	343.29	1552.7	343.19
1559.6	343.19	1568.6	343.19	1575.4	340.09	1578.4	338.59
1582.5	339.39	1587.5	339.39	1592.2	339.49	1595.2	340.39
1602.1	342.39	1606.7	344.89	1612.5	344.99	1642.5	344.49
1672.5	345.19	1702.5	346.39	1732.5	348.79	1762.5	351.39
1862.5	360.29						

X-Y MAX-MIN POINTS:

XMIN	Y	X	YMIN	XMAX	Y	X	YMAX
252.5	370.29	1578.4	338.59	1862.5	360.29	252.5	370.29

UBAREA BREAKPOINTS (NSA = 5):

1510. 1548. 1569. 1607.

ROUGHNESS COEFFICIENTS (NSA = 5):

.047 .031 .150 .031 .150

BRIDGE PROJECTION DATA: XREFLT XREFRT FDSTLT FDSTRT

MSPRO
060188

FEDERAL HIGHWAY ADMINISTRATION - U. S. GEOLOGICAL SURVEY
MODEL FOR WATER-SURFACE PROFILE COMPUTATIONS
(Input modified to free format by GKY&A 01/92)

STR. NO. 127020100100 CHESTER CO.
S-201 OVER LITTLE SANDY RIVER

*** RUN DATE & TIME: 02-15-95 09:28

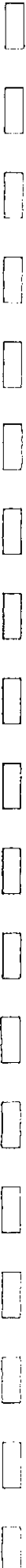
CROSS-SECTION PROPERTIES: ISEQ = 5; SECID = APPR ; SRD = 1153.

WSEL	SA#	AREA	K	TOPW	WETP	ALPH	LEW	REW	QCR
	1	460.	13091.	540.	540.				2408.
	2	166.	20969.	37.	39.				1980.
	3	31.	404.	21.	21.				216.
	4	153.	18070.	38.	40.				1751.
	5	2.	4.	20.	20.				4.
344.69		812.	52539.	657.	660.	2.72	961.	1651.	3108.

HP

1 APPR 347.41

* 347.41



WSPRO
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FEDERAL HIGHWAY ADMINISTRATION - U. S. GEOLOGICAL SURVEY
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CROSS-SECTION PROPERTIES: ISEQ = 5; SECID = APPR ; SRD = 1153.

WSEL	SA#	AREA	K	TOPW	WETP	ALPH	LEW	REW	QCR
	1	2355.	147869.	845.	845.				22314.
	2	267.	46560.	37.	39.				4059.
	3	88.	2287.	21.	21.				1028.
	4	257.	42446.	38.	40.				3783.
	5	226.	3672.	109.	109.				1857.
347.41		3194.	242833.	1050.	1054.	2.25	665.	1715.	21083.

HP

1 APPR 348.16

* 348.16



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CROSS-SECTION PROPERTIES: ISEQ = 5; SECID = APPR ; SRD = 1153.

WSEL	SA#	AREA	K	TOPW	WETP	ALPH	LEW	REW	QCR
	1	3019.	210393.	926.	926.				30932.
	2	296.	54980.	37.	39.				4713.
	3	104.	3006.	21.	21.				1315.
	4	285.	50607.	38.	40.				4431.
	5	311.	5907.	118.	118.				2872.
348.16		4016.	324892.	1141.	1145.	2.13	584.	1725.	29323.

HP

2 APPR 344.69

* 344.69 1750



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STR. NO. 127020100100 CHESTER CO.
S-201 OVER LITTLE SANDY RIVER
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VELOCITY DISTRIBUTION: ISEQ = 5; SECID = APPR ; SRD = 1153.

	WSEL	LEW	REW	AREA	K	Q	VEL
	344.69	960.9	1651.0	812.0	52539.	1750.	2.16
STA.	960.9		1170.6	1249.6	1315.8	1371.5	1510.6
A(I)		130.1		80.1	76.9	74.8	99.0
V(I)		.67		1.09	1.14	1.17	.88
X STA.	1510.6		1517.6	1521.6	1525.4	1528.7	1531.9
A(I)		25.1		19.2	19.1	18.0	18.1
V(I)		3.48		4.56	4.59	4.87	4.85
X STA.	1531.9		1535.6	1539.9	1554.0	1576.0	1579.6
A(I)		18.6		19.6	36.4	45.6	20.0
V(I)		4.71		4.46	2.40	1.92	4.37
STA.	1579.6		1582.8	1586.3	1589.9	1593.5	1651.0
A(I)		18.2		18.6	18.5	18.6	37.6
V(I)		4.80		4.71	4.72	4.71	2.33

HP

2 APPR 347.41

* 347.41 6950



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FEDERAL HIGHWAY ADMINISTRATION - U. S. GEOLOGICAL SURVEY
MODEL FOR WATER-SURFACE PROFILE COMPUTATIONS
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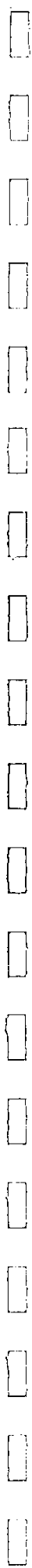
VELOCITY DISTRIBUTION: ISEQ = 5; SECID = APPR ; SRD = 1153.

	WSEL	LEW	REW	AREA	K	Q	VEL
	347.41	665.5	1715.2	3194.1	242833.	6950.	2.18
STA.	665.5		1006.7	1060.0	1109.9	1154.7	1196.2
A(I)		536.1		175.1	170.7	161.1	156.2
V(I)		.65		1.98	2.04	2.16	2.22
X STA.	1196.2		1240.0	1280.3	1321.1	1357.6	1396.4
A(I)		161.8		156.0	157.5	149.3	152.9
V(I)		2.15		2.23	2.21	2.33	2.27
X STA.	1396.4		1443.6	1504.3	1519.7	1527.9	1535.8
A(I)		167.6		184.2	88.5	64.4	63.5
V(I)		2.07		1.89	3.93	5.40	5.48
STA.	1535.8		1547.0	1578.2	1586.0	1594.1	1715.2
A(I)		75.1		153.0	64.1	64.0	293.1
V(I)		4.63		2.27	5.42	5.43	1.19

HP

2 APPR 348.16

* 348.16 10322



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FEDERAL HIGHWAY ADMINISTRATION - U. S. GEOLOGICAL SURVEY
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S-201 OVER LITTLE SANDY RIVER
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VELOCITY DISTRIBUTION: ISEQ = 5; SECID = APPR ; SRD = 1153.

	WSEL	LEW	REW	AREA	K	Q	VEL
	348.16	584.0	1724.6	4015.5	324892.	10322.	2.57
STA.	584.0		977.8	1032.7	1079.9	1127.2	1170.6
A(I)		713.7		212.7	194.2	199.4	191.7
V(I)		.72		2.43	2.66	2.59	2.69
X STA.	1170.6		1212.3	1253.8	1293.2	1333.8	1368.4
A(I)		188.6		184.8	184.6	186.9	169.4
V(I)		2.74		2.79	2.80	2.76	3.05
X STA.	1368.4		1410.0	1458.7	1511.0	1522.8	1531.7
A(I)		189.3		203.9	205.1	90.0	79.1
V(I)		2.73		2.53	2.52	5.74	6.53
Y STA.	1531.7		1541.9	1575.9	1585.0	1594.5	1724.6
A(I)		83.3		191.0	81.8	82.0	384.2
V(I)		6.20		2.70	6.31	6.30	1.34

EX

+++ BEGINNING PROFILE CALCULATIONS -- 3

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FEDERAL HIGHWAY ADMINISTRATION - U. S. GEOLOGICAL SURVEY
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XSID:CODE	SRDL	LEW	AREA	VHD	HF	EGL	CRWS	Q	WSEL
SRD	FLEN	REW	K	ALPH	HO	ERR	FR#	VEL	
XIT :XS	*****	999.	590.	.34	*****	344.15	342.41	1750.	343.81
880.	*****	1606.	40124.	2.48	*****	*****	.81	2.97	

===125 FR# EXCEEDS FNTEST AT SECID "FULLV": TRIALS CONTINUED.
FNTEST,FR#,WSEL,CRWS = .80 .80 344.04 342.63

===110 WSEL NOT FOUND AT SECID "FULLV": REDUCED DELTAY.
WSLIM1,WSLIM2,DELTAY = 343.31 370.00 .50

===115 WSEL NOT FOUND AT SECID "FULLV": USED WSMIN = CRWS.
WSLIM1,WSLIM2,CRWS = 343.31 370.00 342.63

FULLV:FV	120.	997.	601.	.33	.23	344.38	342.63	1750.	344.05
1000.	120.	1606.	40696.	2.50	.00	.01	.79	2.91	

<<<<THE ABOVE RESULTS REFLECT "NORMAL" (UNCONSTRICTED) FLOW>>>>

APPR :AS	153.	998.	598.	.33	.28	344.67	*****	1750.	344.34
1153.	153.	1606.	40570.	2.49	.00	.00	.80	2.92	

<<<<THE ABOVE RESULTS REFLECT "NORMAL" (UNCONSTRICTED) FLOW>>>>

<<<<RESULTS REFLECTING THE CONSTRICTED FLOW FOLLOW>>>>

XSID:CODE	SRDL	LEW	AREA	VHD	HF	EGL	CRWS	Q	WSEL
SRD	FLEN	REW	K	ALPH	HO	ERR	FR#	VEL	
BRID :BR	120.	1499.	312.	.64	.28	344.54	342.71	1750.	343.90
1000.	120.	1605.	33107.	1.30	.11	.00	.66	5.61	

TYPE PPCD FLOW C P/A LSEL BLEN XLAB XRAB
3. 1. 1. .878 .027 346.50 ***** ***** *****

XSID:CODE SRD FLEN HF VHD EGL ERR Q WSEL
ROAD :RG 1017. <<<<EMBANKMENT IS NOT OVERTOPPED>>>>

XSID:CODE	SRDL	LEW	AREA	VHD	HF	EGL	CRWS	Q	WSEL
SRD	FLEN	REW	K	ALPH	HO	ERR	FR#	VEL	
PPR :AS	120.	961.	810.	.20	.24	344.88	342.93	1750.	344.69
1153.	136.	1651.	52446.	2.72	.11	.02	.56	2.16	

M(G) M(K) KQ XLKQ XRKQ OTEL
.823 .238 39517. 1493. 1599. 344.55

<<<<END OF BRIDGE COMPUTATIONS>>>>

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FEDERAL HIGHWAY ADMINISTRATION - U. S. GEOLOGICAL SURVEY
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XSID:CODE	SRDL	LEW	AREA	VHD	HF	EGL	CRWS	Q	WSEL
SRD	FLEN	REW	K	ALPH	HO	ERR	FR#	VEL	
XIT :XS	*****	766.	2274.	.36	*****	346.32	345.14	6950.	345.96
880.	*****	1704.	159403.	2.45	*****	*****	.54	3.06	

FULLV:FV	120.	765.	2287.	.35	.23	346.56	*****	6950.	346.21
1000.	120.	1704.	160505.	2.45	.00	.01	.54	3.04	

<<<<<THE ABOVE RESULTS REFLECT "NORMAL" (UNCONSTRICTED) FLOW>>>>>

PPR :AS	153.	764.	2295.	.35	.29	346.85	*****	6950.	346.51
1153.	153.	1704.	161206.	2.44	.00	.01	.53	3.03	

<<<<<THE ABOVE RESULTS REFLECT "NORMAL" (UNCONSTRICTED) FLOW>>>>>

===215 FLOW CLASS 1 SOLUTION INDICATES POSSIBLE ROAD OVERFLOW.
WS1,WSSD,WS3,RGMIN = 350.93 .00 346.21 344.70

===260 ATTEMPTING FLOW CLASS 4 SOLUTION.

===220 FLOW CLASS 1 (4) SOLUTION INDICATES POSSIBLE PRESSURE FLOW.
WS3,WSIU,WS1,LSEL = 346.74 347.19 347.64 346.50

===245 ATTEMPTING FLOW CLASS 2 (5) SOLUTION.

<<<<<RESULTS REFLECTING THE CONSTRICTED FLOW FOLLOW>>>>>

XSID:CODE	SRDL	LEW	AREA	VHD	HF	EGL	CRWS	Q	WSEL
SRD	FLEN	REW	K	ALPH	HO	ERR	FR#	VEL	
BRID :BR	120.	1493.	607.	.58	*****	347.77	344.36	3455.	347.19
1000.	*****	1612.	58457.	1.15	*****	*****	.48	5.69	

TYPE PPCD FLOW	C	P/A	LSEL	BLEN	XLAB	XRAB
3. 1. 5.	.391	.025	346.50	*****	*****	*****

XSID:CODE	SRD	FLEN	HF	VHD	EGL	ERR	Q	WSEL
ROAD :RG	1017.	120.	.10	.17	347.47	.00	3501.	347.01

	Q	WLEN	LEW	REW	DMAX	DAVG	VMAX	VAVG	HAVG	CAVG
LT:	3427.	548.	909.	1457.	2.3	1.2	5.8	5.4	1.6	3.0
RT:	74.	45.	1718.	1763.	.4	.2	3.7	8.0	.7	3.0

XSID:CODE	SRDL	LEW	AREA	VHD	HF	EGL	CRWS	Q	WSEL
SRD	FLEN	REW	K	ALPH	HO	ERR	FR#	VEL	
APPR :AS	120.	666.	3190.	.17	.30	347.57	345.66	6950.	347.41
1153.	160.	1715.	242441.	2.25	.05	.00	.33	2.18	

M(G)	M(K)	KQ	XLKQ	XRKQ	OTEL
*****	*****	*****	*****	*****	*****

<<<<<END OF BRIDGE COMPUTATIONS>>>>>

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FEDERAL HIGHWAY ADMINISTRATION - U. S. GEOLOGICAL SURVEY
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XSID:CODE	SRDL	LEW	AREA	VHD	HF	EGL	CRWS	Q	WSEL
SRD	FLEN	REW	K	ALPH	HO	ERR	FR#	VEL	
XIT :XS	*****	672.	3129.	.38	*****	347.21	345.70	10322.	346.83
880.	*****	1714.	236626.	2.26	*****	*****	.50	3.30	

ULLV:FV	120.	670.	3146.	.38	.23	347.45	*****	10322.	347.07
1000.	120.	1715.	238266.	2.26	.00	.01	.50	3.28	

<<<<THE ABOVE RESULTS REFLECT "NORMAL" (UNCONSTRICTED) FLOW>>>>

PPR :AS	153.	669.	3158.	.37	.29	347.75	*****	10322.	347.38
1153.	153.	1715.	239328.	2.26	.00	.01	.50	3.27	

<<<<THE ABOVE RESULTS REFLECT "NORMAL" (UNCONSTRICTED) FLOW>>>>

===255 ATTEMPTING FLOW CLASS 3 (6) SOLUTION.
WS3N,LSEL = 347.07 346.50

<<<<RESULTS REFLECTING THE CONSTRICTED FLOW FOLLOW>>>>

XSID:CODE	SRDL	LEW	AREA	VHD	HF	EGL	CRWS	Q	WSEL
SRD	FLEN	REW	K	ALPH	HO	ERR	FR#	VEL	
PRID :BR	120.	1493.	605.	.69	*****	347.76	344.56	3705.	347.07
1000.	*****	1612.	61023.	1.18	*****	*****	.52	6.12	

TYPE	PPCD	FLOW	C	P/A	LSEL	BLEN	XLAB	XRAB
3.	1.	6.	.800	.025	346.50	*****	*****	*****

XSID:CODE	SRD	FLEN	HF	VHD	EGL	ERR	Q	WSEL
ROAD :RG	1017.	120.	.12	.22	348.26	.02	6816.	347.67

	Q	WLEN	LEW	REW	DMAX	DAVG	VMAX	VAVG	HAVG	CAVG
LT:	6397.	703.	787.	1491.	3.0	1.5	6.6	6.1	2.1	3.0
RT:	420.	116.	1683.	1799.	1.1	.5	4.8	6.8	1.1	3.0

XSID:CODE	SRDL	LEW	AREA	VHD	HF	EGL	CRWS	Q	WSEL
SRD	FLEN	REW	K	ALPH	HO	ERR	FR#	VEL	
APPR :AS	120.	584.	4017.	.22	.40	348.38	346.22	10322.	348.16
1153.	165.	1725.	325069.	2.13	.05	.02	.35	2.57	

M(G)	M(K)	KQ	XLKQ	XRKQ	OTEL
*****	*****	*****	*****	*****	*****

<<<<END OF BRIDGE COMPUTATIONS>>>>

ER



=====

STRUCTURE NO. 127020100100 S-201 OVER LITTLE SANDY RIVER CHESTER CO.
100 YEAR CHANNEL CONTRACTION SCOUR

Y1= 8.8 ft
Wc1= 38 ft
Wc2= 38 ft
Qmc1= 1215 cfs
Qmc2= 1614 cfs
K1= 0.64

Y2= $Y1 * (Qmc2/Qmc1)^{(6/7)} * (Wc1/Wc2)^{K1}$
Y2= 11.3 ft
Ys=Y2-Y1= 2.4 ft

500 YEAR CHANNEL CONTRACTION SCOUR

Y1= 9.6 ft
Wc1= 38 ft
Wc2= 38 ft
Qmc1= 1608 cfs
Qmc2= 1817 cfs
K1= 0.64

Y2= $Y1 * (Qmc2/Qmc1)^{(6/7)} * (Wc1/Wc2)^{K1}$
Y2= 10.6 ft
Ys=Y2-Y1= 1.1 ft

100 YEAR CHANNEL PIER SCOUR

Y1= 8.89
K1= 1.1
K2= 1.0
K3= 1.1
a= 0.8 ft
V1= 7.0
Fr1= 0.416

Ys= $Y1 * 2 * K1 * K2 * K3 * (a/Y1)^{0.65} * Fr1^{0.43}$
Ys= 3.2

500 YEAR CHANNEL PIER SCOUR

Y1= 8.8
K1= 1.1
K2= 1.0
K3= 1.1
a= 0.8 ft
V1= 9.28
Fr1= 0.552

Ys= $Y1 * 2 * K1 * K2 * K3 * (a/Y1)^{0.65} * Fr1^{0.43}$
Ys= 3.6

=====

STRUCTURE NO. 127020100100 S-201 OVER LITTLE SANDY RIVER CHESTER CO.
OVERTOPPING CHANNEL CONTRACTION SCOUR

Y1= 6.1 ft
Wc1= 38 ft
Wc2= 38 ft
Qmc1= 602 cfs
Qmc2= 784 cfs
K1= 0.64

Y2= $Y1 * (Qmc2 / Qmc1)^{(6/7)} * (Wc1 / Wc2)^{K1}$
Y2= 7.7 ft
Ys=Y2-Y1= 1.6 ft

OVERTOPPING CHANNEL PIER SCOUR

Y1= 5.6
K1= 1.1
K2= 1.0
K3= 1.1
a= 0.8 ft
V1= 7.2
Fr1= 0.535

Ys= $Y1 * 2 * K1 * K2 * K3 * (a / Y1)^{0.65} * Fr1^{0.43}$
Ys= 3.0

STRUCTURE NO. 127020100100 S-201 OVER LITTLE SANDY RIVER CHESTER CO.
OVERTOPPING SOUTHERN ABUTMENT SCOUR

a' LENGTH OF ABUT.

PROJ. NORMAL TO FLOW= 549.0
OBSTRUCTED FLOW AREA= 460.9
Ya= 0.8

VERT. WALL 1
VERT. WALL W/ WINGS 0.82
SPILL THROUGH 0.55
K1= 0.55

EMB. ANGLE (PERP.=90) 90 SIDE WITH FLOW REVERSAL HAS HIGHER ANGLE
K2= 1.000

OBSTRUCTED FLOW (cfs) 436.8
Ve= 0.95
Fre=Ve/(gYa)^{0.5}= 0.1823

Ys/Ya=2.27 K1 K2 (a'/Ya)^{0.43} Fre^{0.61} + 1
Ys= 7.0

OVERTOPPING NORTHERN ABUTMENT SCOUR

a' LENGTH OF ABUT.

PROJ. NORMAL TO FLOW= 58.0
OBSTRUCTED FLOW AREA= 38
Ya= 0.6

VERT. WALL 1
VERT. WALL W/ WINGS 0.82
SPILL THROUGH 0.55
K1= 0.55

EMB. ANGLE (PERP.=90) 90 SIDE WITH FLOW REVERSAL HAS HIGHER ANGLE
K2= 1.000

OBSTRUCTED FLOW (cfs) 88
Ve= 2.33
Fre=Ve/(gYa)^{0.5}= 0.5100

Ys/Ya=2.27 K1 K2 (a'/Ya)^{0.43} Fre^{0.61} + 1
Ys= 4.7

APPENDIX 5

Selected References

1. HEC-20, Stream Stability at Highway Structures, Publication No. FHWA-IP-90-014. FHWA, U.S. Department of Transportation, February 1991
2. Techniques for Estimating Magnitude and Frequency of Floods in South Carolina, 1988, U.S. Geological Survey Water-Resources Investigations Report 91-4157.
3. The WSPRO computer program and manual, prepared by the U.S. Geological Survey for The Federal Highway Administration.
4. HEC-18, Evaluating Scour at Bridges (Second Edition), Publication No. FHWA-IP-90-017. FHWA, U.S. Department of Transportation, Revised April 1993.
5. Magnitude and Frequency of Floods in Rural and Urban Basins of North Carolina, 1987, U.S. Geological Survey, Water-Resources Investigations Report 87-4096.