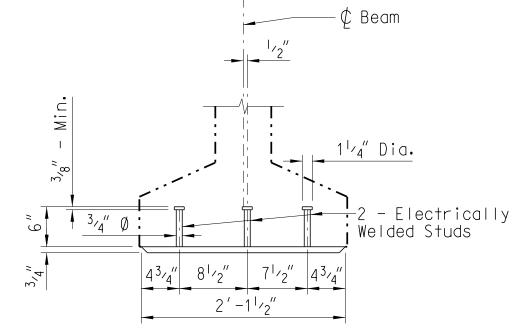


UA1601 R1001 R1001 UA1601 R1001 R1001 End of Beam 3 - Electrically Welded Studs

Bottom of prestr. conc. beam and bottom of sole PL

ELEV. AT SIDE OF BEAM



ELEV. AT END OF BEAM

END VIEW

Showing UA1601 & UA2901 bars

SOLE PLATE DETAIL

BEA	M CA	MBER	AND DE	FLECTI	ON	
BEAM CAMBER		DEFLECTION DUE TO				
AT RELEASE	* AT ERECTION	INTERIOR DIAPHRAGM	STAY-IN-PLACE FORMS**	SLAB	BARRIER PARAPET	
Χ"	Χ"	Χ"	Χ"	Χ"	Χ"	

* Based on a beam age of 60 days at the time of erection

** Deflection due to the weight of the metal forms and the weight of the concrete in the valleys of the forms.

"+" indicates upward movement

Note to Designer:

| Note to Designer:
| This drawing is furnished for information only. All dimensions shown are sheet | specific. Any use of this design and drawing, including dimensions, must be | specific. Any use of this design and drawing is adequate for the intended use. | checked by the User's Engineer to ensure design is adequate for the intended use. | All drawings must be signed and sealed by a South Carolina Registered Professional | Engineer when used. | If plans are detailed with Steel Intermediate Diaphragms, revise beam details for | If plans are detailed with Steel Intermediate Diaphragms, revise beam details for | If plans are detailed with Steel Intermediate Diaphragms.

"-" indicates downward movement					
	TOLER	ANCES			
Q ₁	10'-0"			S3 83	
do	<u> </u>			<u> </u>	
d h	PLAN	ກ → S ₁	p	(∋
		^ <u> </u>	O	<u>b</u> 1	
d khead; gle					₹ <u>2</u>
<u>e</u> <u>k</u> 1			m ₁	00000	00000
	ELEVATION			CROSS S	BECTION

а	Length	$\pm \frac{1}{4}$ " per 25' length, \pm 1" max.
b	Width (overall)	+ ³ / ₈ ", - ¹ / ₄ "
b ₁	Web Width	+ 3/8", - 1/4"
С	Depth (overall)	+ 1/2", - 1/4"
C ₁	Flange Depth	± 1/4"
d	Variation from Specified Plan End Squareness or Skew	$\pm \frac{1}{8}$ per 12" width, $\pm \frac{1}{2}$ " max.
е	Variation from Specified Elevation End Squareness or Skew	\pm 3 / $_{16}$ " per 12" depth, \pm 1" max.
f	Sweep	1/8" per 10' length
g	Camber Variation from Design Camber (measurement of camber for comparison to predicted design values should be completed within 72 hrs. of transfer of prestr. force)	$\pm \frac{1}{8}$ " per 10' $\pm \frac{1}{2}$ " max. up to 80' length \pm 1" max. for length greater than 80'
h	Local Smoothness of Any Surface	' ₄ " in 10'
k	Location of Strand (Individual)	± 1/4"
	Location of Strand (Bundled)	± 1/2"
K ₁	Location of Harp Points for Harped Strands from Design Location	± 20"
k ₂	Location of Post-Tensioning Duct	± 1/4"
1	Location of Embedment	± 1"
12	Tipping and Flushness of Embedment	± 1/4"
m_1	Location of Bearing Assembly	± 5/8"
m_2	Tipping and Flushness of Bearing Assembly	± 1/8"
Р	Location of Inserts, Sleeves, or Holes for Structural Connections	± 1/2"
91	Location of Handling Device Parallel to Length of Member	± 6"
q ₂	Location of Handling Device Transverse to Length of Member	± 1"
S ₁	Longitudinal Spacing of Stirrups	± 2"
	Longitudinal Spacing of Stirrups within Distance "c" from Member Ends	± 1"
S3	Stirrup Projection from Beam Surface	+ 1/4", - 1/2"
S ₄	Reinforcing Bar Projection from Beam End	± 1/2"

Notes:

See Section 704 of the Standard Specifications for additional requirements and information regarding prestressed concrete beams. Shop drawings must be submitted in accordance with the Standard Specifications.

All overhang brackets in the top flange of exterior beams shall be galvanized in accordance with AASHTO M 111, AASHTO M 232, or ASTM F 2329 as appropriate and shall be detailed accordingly in the shop plans.

Use prestressing strands that conform to the latest AASHTO M 203 for grade 270 (low relaxation).

The tensioning load in all X" Dia. low relaxation strands is XX.X kips. Do not release the strands until the compressive strength of the concrete has reached the value shown for f'ci.

On the top surface of beams where cast-in-place concrete will be placed, provide a finish that is clean, free of laitance, and intentionally roughened to a full amplitude of approximately $\frac{1}{4}$. Finish top of beam level.

Always maintain prestressed concrete beams in an upright position. Use lifting devices provided at each end of the beam to lift or handle beams. Do not permit beams to be placed or stored on interior supports causing negative moments.

Locate holes for tie bars as shown on this drawing. Form holes with 2" inside dia. pipe and prevent movement during casting by securely fastening the pipe.

Debonding:

1) For all debonding material, use tubular conduit capable of resisting the pressure exerted by the concrete. When using slit conduit, use two conduits with the slits located on opposite sides of the strand. Use conduit made of high density polyethylene or polyprophylene with a minimum thickness of 0.025". Use conduit with an inside diameter that will permit free movement of the encased strand, but no larger than the diameter of the strand plus 1/8". Place conduit on the strand at the location(s) shown on the plans (+/- 1") to prevent bonding of the concrete. Secure conduit to prevent any longitudinal movement along the strand. Prevent concrete from entering the conduit by sealing with tape. Use tape manufactured from a non-corrosive material that is compatible with the concrete, conduit, and steel.

2) Release strands in accordance with Section 704 of the Standard Specifications.

3) Within 48 hours of detensioning, seal the openings between the strands and the sheathing. Use an approved sealant that is made of either epoxy or silicone. If silicone sealant is provided, use a low modulus silicone sealant that is white in color.

For additional notes and details see Sh. XX.

REV. PCW HL 3-19
Debonding Note
REV. PCW HL 3-19
Debonding Note
REV. PCW HL 3-19
Sole PL Detail
REVIEWED

OUAN. DR. PNP SAN 6-08

BY CHK. DATE

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DEPARTMENT OF TRANSPORTATION
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