

Table 4.2. Summary of Tidal Hydraulics at Upstream Face of Proposed SC 73 Bridge Over Withers Swash.

Return Period	=	50 years (approximate incipient overtopping event)
Riverine Hydrology Data		
Effective Drained Area	=	2.2 mi ²
50-Year Discharge	=	1,654 cfs
Tidal Event Hydrology		
50-Year Maximum Discharge	=	1,275 cfs
50-Year WSEL	=	9.38 ft-NAVD
50-Year Average Velocity in Bridge Opening	=	2.55 ft/sec
Simulation Time Period	=	121.75
Tidal Event and Riverine Events Combined		
50-Year Peak Discharge	=	2,228 cfs
50-Year WSEL at Peak Discharge	=	10.5 ft-NAVD
50-Year Channel Velocity in Bridge Opening	=	4.4 ft/sec
Simulation Time Period	=	124 hrs

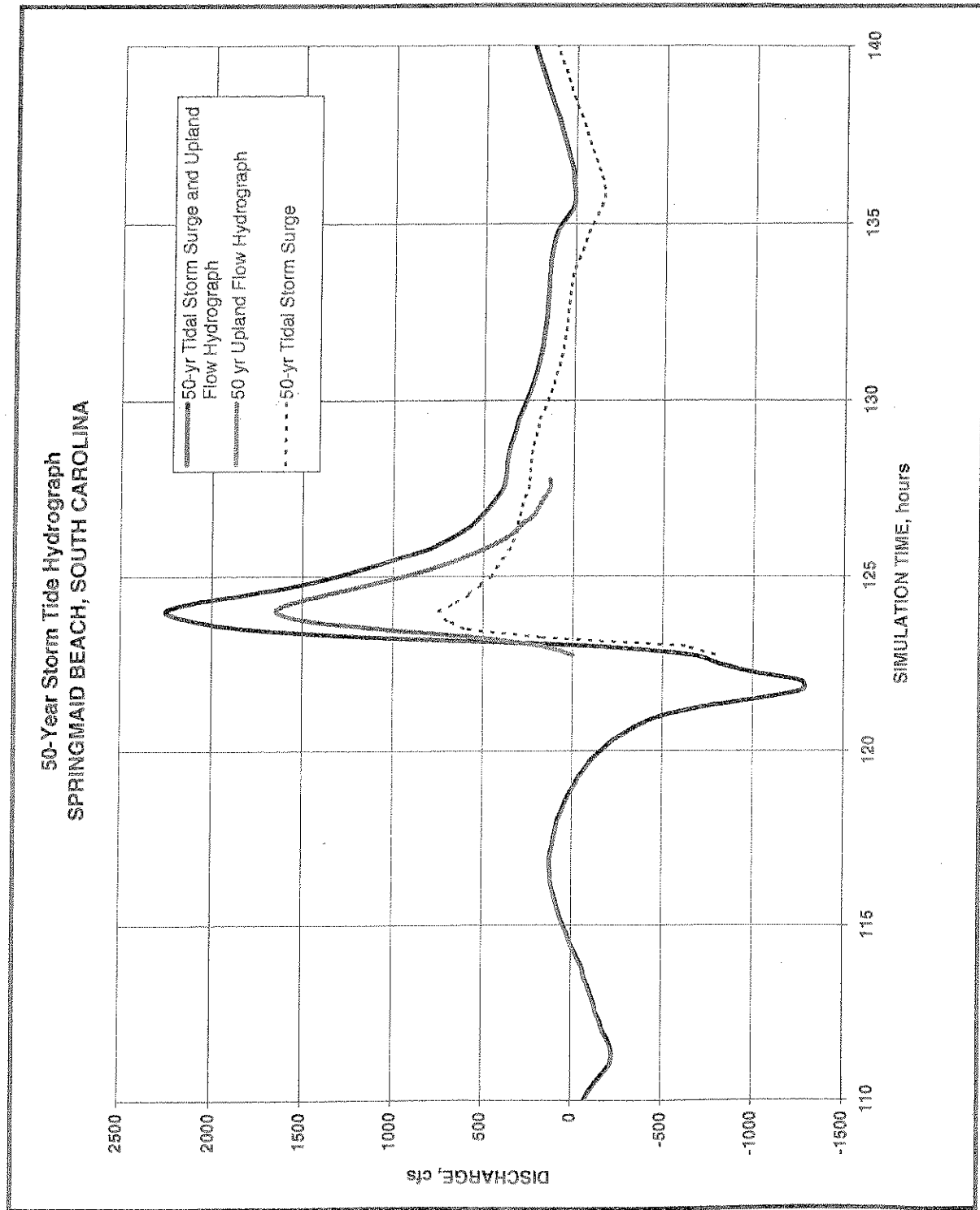
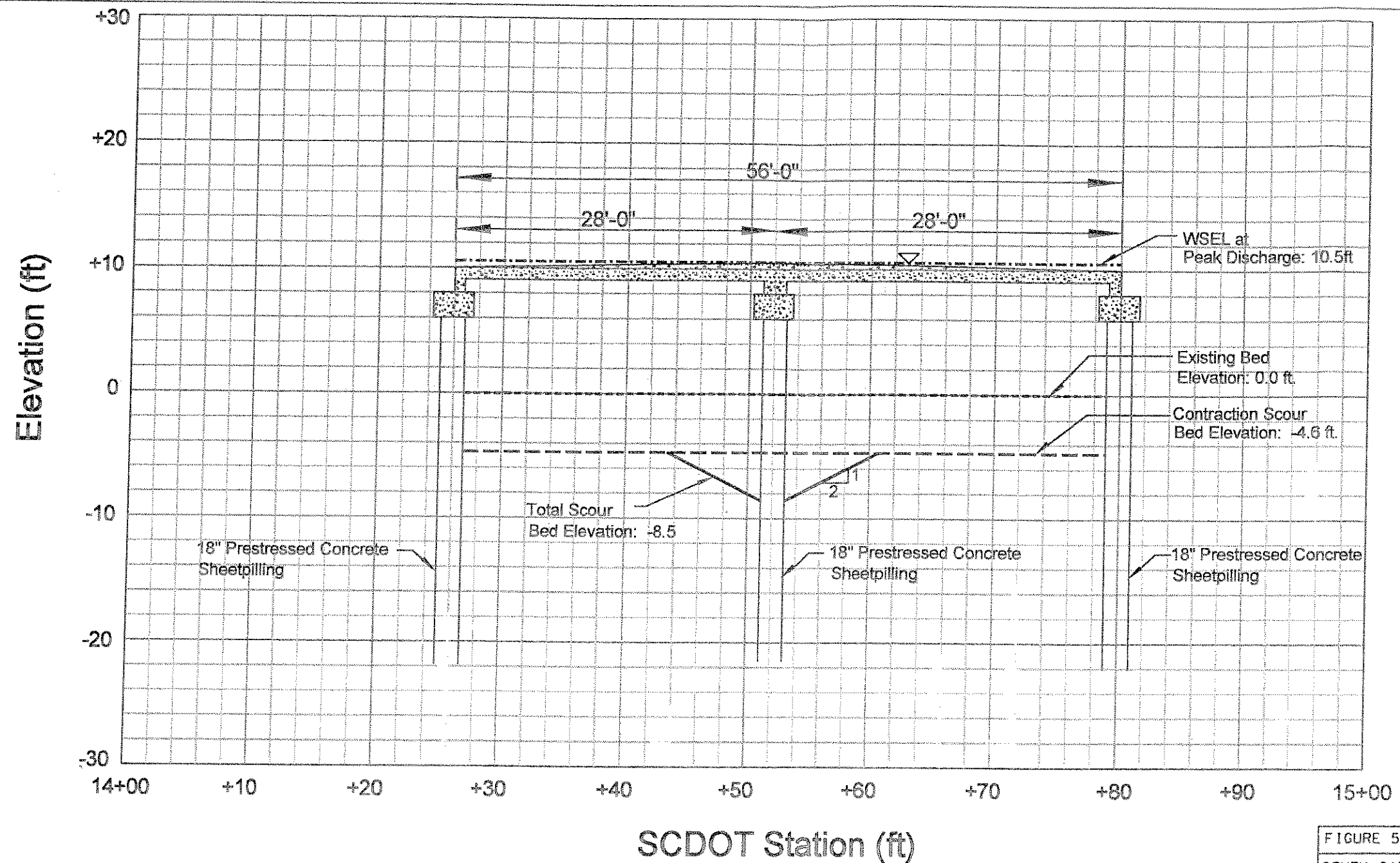


Figure 4.2. Tidal surge and upland flow hydrographs.



HYDRAULIC INFORMATION
50 YEAR STORM SURGE AND 50 YEAR RIVERINE DISCHARGE

EFFECTIVE DRAINED AREA (SQ. MILES)	= 2.2
Q 50 (CFS)	= 2228
50-YEAR WSEL (FT-NAVD)	= 10.5
AVERAGE VELOCITY AT BRIDGE OPENING	= 4.4
INCIPIENT OVERTOPPING FLOOD RECURRENCE INTERVAL	= 50 YR
OVERTOPPING FLOOD PROBABILITY	= 0.02

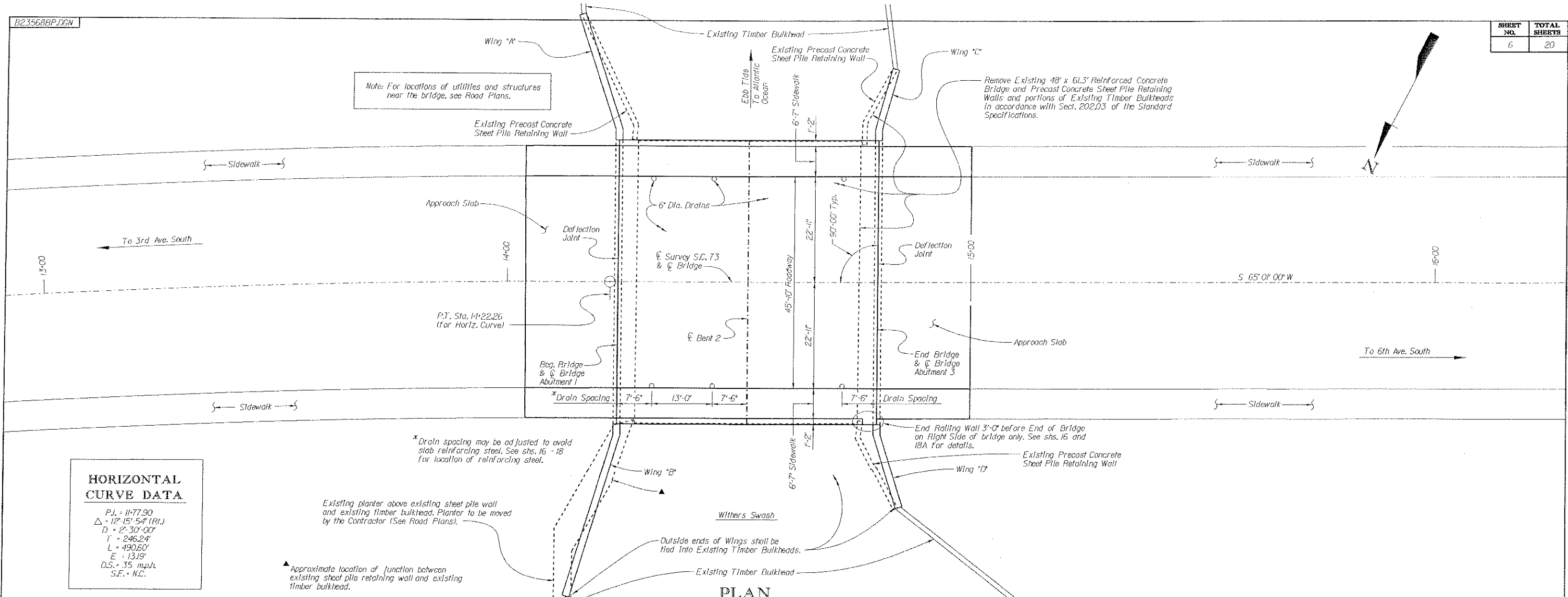
FIGURE 5.1 FEBRUARY 2000
SOUTH CAROLINA DEPARTMENT OF TRANSPORTATION
HORRY COUNTY, SOUTH CAROLINA

SCOUR PROFILE
SC 73 OVER WITHERS SWASH

AYRES ENGINEERS, SCIENTISTS & SURVEYORS
3665 JFK Pkwy., Bldg. 2, Ste. 300
Ft. Collins, CO 80525
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Figure 5.1. Scour profile.

SHEET NO.	TOTAL SHEETS
6	20



Note: For locations of utilities and structures near the bridge, see Road Plans.

Remove Existing 48' x 61.3' Reinforced Concrete Bridge and Precast Concrete Sheet Pile Retaining Walls and portions of Existing Timber Bulkheads in accordance with Sect. 202.03 of the Standard Specifications.

HORIZONTAL CURVE DATA

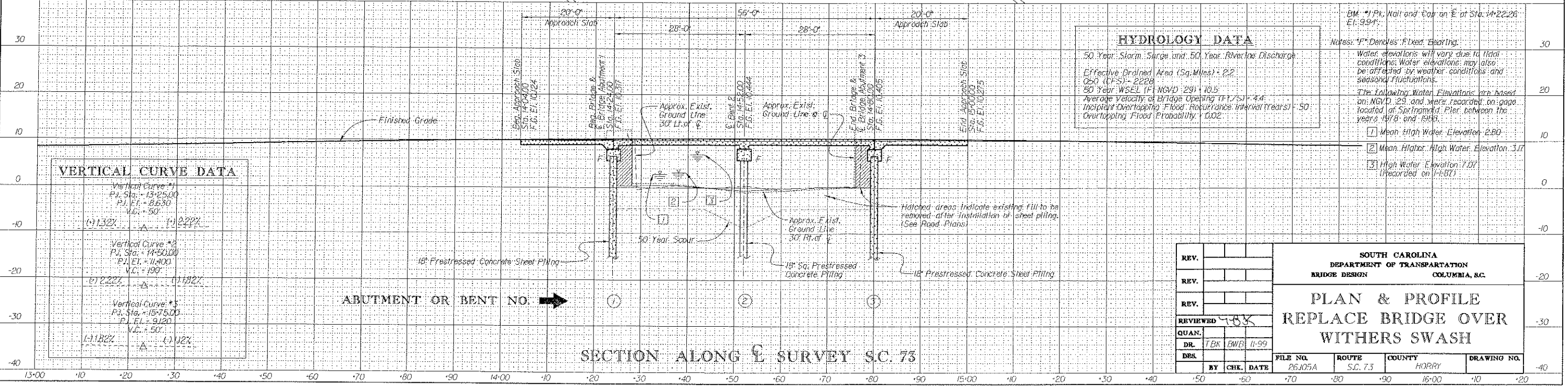
P.I. = 1477.90
 Δ = 12° 15' 54" (RT.)
 D = 2° 30' 00"
 L = 246.24'
 L = 490.60'
 E = 13.19'
 O.S. = 35 mph
 S.E. = N.C.

Existing planter above existing sheet pile wall and existing timber bulkhead. Planter to be moved by the Contractor (See Road Plans).

Approximate location of junction between existing sheet pile retaining wall and existing timber bulkhead.

* Drain spacing may be adjusted to avoid slab reinforcing steel. See shs. 16 - 18 for location of reinforcing steel.

PLAN



VERTICAL CURVE DATA

Vertical Curve #1
 P.I. Sta. = 13+25.00
 P.L. El. = 86.30
 V.C. = 50'
 (+) 1.32% (-) 2.22%

Vertical Curve #2
 P.I. Sta. = 14+50.00
 P.L. El. = 11.400
 V.C. = 199'
 (-) 2.22% (-) 1.92%

Vertical Curve #3
 P.I. Sta. = 15+75.00
 P.L. El. = 9.120
 V.C. = 50'
 (-) 1.82% (-) 1.02%

HYDROLOGY DATA

50 Year Storm Surge and 50 Year Riverine Discharge
 Effective Drained Area (Sq. Miles) = 2.2
 O50 (CF5) = 2228
 50 Year WSEL (FT NGVD 29) = 10.5
 Average Velocity of Bridge Opening (FT/S) = 4.4
 Incipient Overtopping Flood Recurrence Interval (Years) = 50
 Overtopping Flood Probability = 0.02

BM #1: Pile Wall and Cap at Sta. 14+22.26
 El. 9.94'

Notes: "F" Denotes Fixed Bearing.

Water elevations will vary due to tidal conditions. Water elevations may also be affected by weather conditions and seasonal fluctuations.

The following Water Elevations are based on: NGVD 29, and were recorded on gage located at Springmaid Pier between the years 1978 and 1998.

- Mean High Water Elevation 2.80'
- Mean Highest High Water Elevation 3.17'
- High Water Elevation 7.07' (recorded on 1-18-87)

REV.				
REV.				
REV.				
REVIEWED	TOS			
QUAN.				
DR.	TBK	EWB	11-99	
DES.				
BY	CHK.	DATE	FILE NO.	ROUTE
			26J05A	S.C. 73
			COUNTY	DRAWING NO.
			HORRY	

SOUTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 BRIDGE DESIGN COLUMBIA, S.C.

PLAN & PROFILE
REPLACE BRIDGE OVER
WITHERS SWASH

ABUTMENT OR BENT NO. →

SECTION ALONG E SURVEY S.C. 73